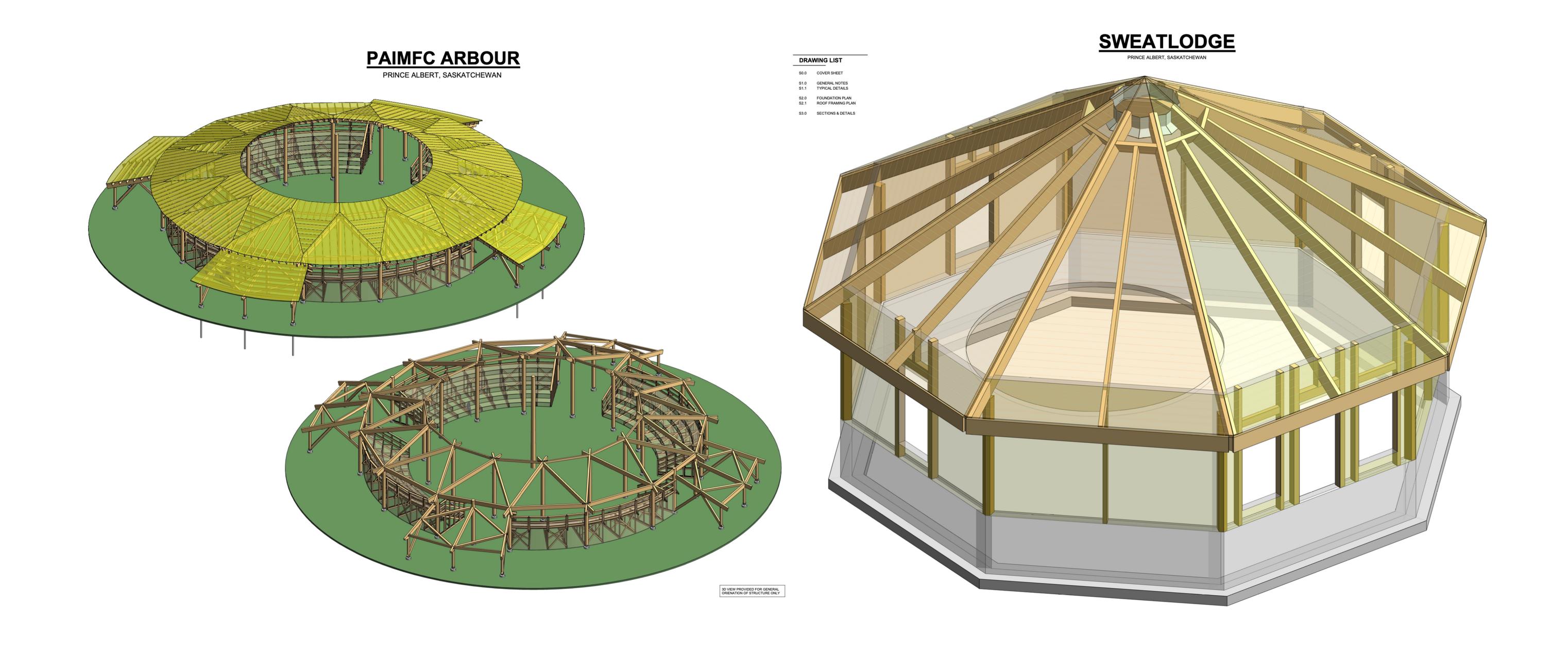
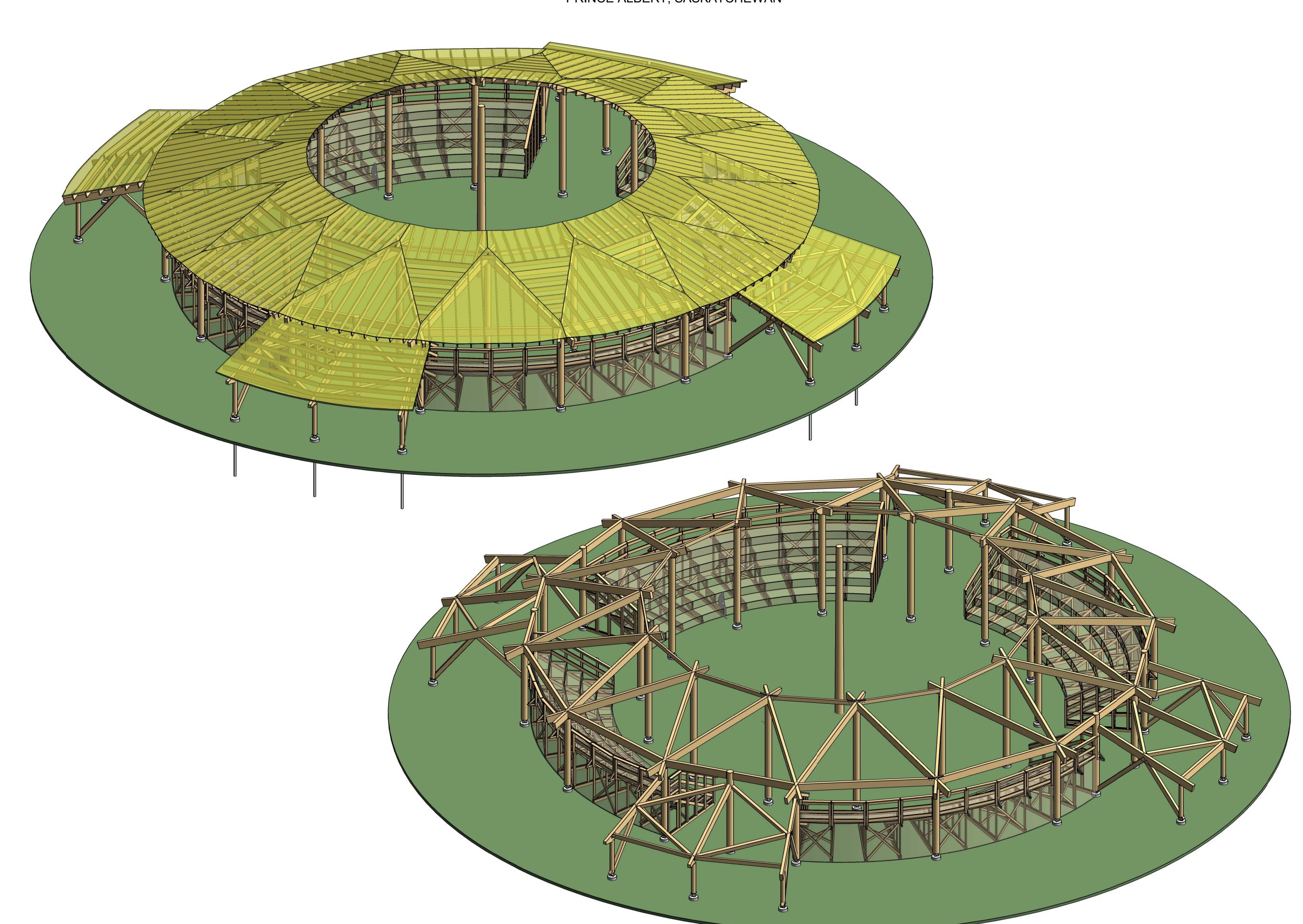
Wakpa Sa Cistinna (Little Red River) Upper Plains Cultural + Ceremonial Grounds



PAIMFC ARBOUR

PRINCE ALBERT, SASKATCHEWAN





619 MAIN STREET SASKATOON, SK S7H 0J8

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Saskatchewan Office 306.280.5588 CHRISTOPHER LAKE, SASKATCHEWAN

P.Eng.

DW 2025.06.18 DW 2025.04.04 ISSUED FOR PERMIT ISSUED FOR REVIEW BY DATE DESCRIPTION

REVISIONS

PAIMFC ARBOUR

PRINCE ALBERT, SASKATCHEWAN

3D VIEW PROVIDED FOR GENERAL ORIENATION OF STRUCTURE ONLY

COVER SHEET

Scale				Revis
Project No.	16918			
Drawn:	TD			Draw
Designed:	DW			
Design Checked:	TH	Date	2024.04.02	Š
Discipline Review:		Date	2024.04.02	
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GENERAL

- 1. THE CONTRACTOR SHALL EXAMINE ALL DRAWINGS, CHECK DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER FOR CLARIFICATION PRIOR TO COMMENCING CONSTRUCTION. DISCREPANCIES NOT REPORTED ARE THE RESPONSIBILITY OF THE CONTRACTOR. CHECK AND VERIFY ALL DIMENSIONS BEFORE COMMENCING WITH ANY WORK. NOTIFY THE ENGINEER OF ANY ERRORS OR OMISSIONS.
- 2. TEMPORARY SUPPORT AND TEMPORARY AND PERMANENT BRACING OF LOAD BEARING AND NON-LOAD BEARING ELEMENTS DURING CONSTRUCTION TO RESIST DEAD, LIVE, ENVIRONMENTAL AND CONSTRUCTION LOADS IS THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN OF THE TEMPORARY SUPPORTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 3. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".

FIELD REVIEWS

- NOTIFY THE ENGINEER 48 HOURS IN ADVANCE FOR FIELD REVIEWS AND APPROVAL OF THE FOLLOWING: * CONCRETE REINFORCEMENT BEFORE EACH CONCRETE POUR * WOOD FRAMING BEFORE COVERING
- 5. DO NOT SCALE FROM THESE DRAWINGS.
- 6. THE DESIGN HAS BEEN PREPARED BASED ON THE ASSUMPTION THAT THE OWNER AND/OR OPERATOR HAS A SITE SAFETY PLAN IN PLACE TO ADDRESS AND MITIGATE SAFETY HAZARDS, BOTH COMMON AND SPECIFIC TO THIS PROJECT.
- 7. LOCATING STRUCTURE ON SITE, EXCAVATION AND COORDINATION WITH UTILITIES IS THE RESPONSIBILITY OF THE OWNER.

TIMBER JOINERY SPECIFICATIONS

- 1. IT IS THE FABRICATOR'S RESPONSIBILITY TO CHOOSE TIMBERS SUITABLE FOR THE APPLICATION AND JOINERY.
- 2. TIMBERS WILL SHRINK AFTER INSTALLATION UNTIL THEY REACH THE EMC (EQUILIBRIUM MOISTURE CONTENT). THIS WILL CAUSE THE LOOSENING OF BOLTS AND THREADED CONNECTORS. PERIODIC TIGHTENING OF THESE CONNECTIONS BY THE END USER DURING THE FIRST SEVERAL YEARS OF THE STRUCTURE'S SERVICE LIFE MAY BE REQUIRED.
- 3. NORMAL CONSTRUCTION TOLERANCES/DIMENSIONAL VARIATIONS CAN BE EXPECTED IN THE COMPLETED STRUCTURE AS WELL AS DIMENSIONAL CHANGES DUE TO SHRINKAGE AND SWELLING THROUGHOUT THE LIFE OF THE STRUCTURE. FOR THESE REASONS, DETAILS OF ADJOINING SURFACES AND MATERIALS MUST BE ABLE TO ACCOMMODATE THESE VARIATIONS AND CHANGES. EXPECT JOINERY TO "OPEN" SOMEWHAT DUE TO SHRINKAGE FROM INITIAL MOISTURE CONTENT AT THE TIME OF FABRICATION UNTIL EMC IS REACHED.
- 4. THE KEY TO THE LONG TERM DURABILITY OF THE STRUCTURE DEPENDS ON KEEPING THE TIMBERS SEALED, DRY AND WELL VENTILATED. THESE DESIGN DETAILS ARE THE RESPONSIBILITY OF

DESIGN DATA

- 1. DESIGN CODE: 2020 NATIONAL BUILDING CODE (NBC).
- 2. MATERIAL DESIGN STANDARDS TO: CSA A23.3-19 DESIGN OF CONCRETE STRUCTURES. CSA S16-14 LIMIT STATES DESIGN OF STEEL STRUCTURES. CSA 086-19 ENGINEERING DESIGN IN WOOD.
- 3. CLIMATIC LOAD DATA AS PER 2020 NBC FOR THE LOCATION OF PRINCE ALBERT
- 4. THE BUILDING IS CONSIDERED NORMAL IMPORTANCE CATEGORY
- 5. SEE PLAN NOTES FOR SPECIFIC LOAD VALUES.
- 6. SELF WEIGHT (SWT) IS DUE TO THE WEIGHT OF THE STRUCTURE
- 7. SUPERIMPOSED DEAD LOADS (SDL) ARE NON-STRUCTURAL DEAD LOADS DUE TO NON-STRUCTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, SUSPENDED EQUIPMENT, PAVERS, SOIL, ETC.
- 8. DEAD LOAD (DL) IS THE SELF WEIGHT OF THE STRUCTURE PLUS THE SUPERIMPOSED DEAD LOAD.
- 9. UNLESS OTHERWISE NOTED, DESIGN LOADS SHOWN ON DRAWINGS ARE SPECIFIED (UNFACTORED) LOADS, TO BE USED FOR ULS DESIGN. FOR SLS DESIGN, THESE LOADS CAN BE REDUCED BY MULTIPLYING WITH THE RATIO OF APPROPRIATE IMPORTANCE FACTORS Ix(SLS) / Ix(ULS) GIVEN BELOW:
- 10. LIVE LOAD 4.8 kPa (BLEACHERS)
- 11. SNOW: Ss = 1.90 kPa , Sr = 0.1 kPa , Is (ULS) = 1.0 , Is (SLS) = 0.9
- 12. RAIN: 24 HOUR RAINFALL = 81 mm
- 13. WIND: q50 = 0.38 kPa, lw(ULS) = 1.0, lw(SLS) = 0.75 BUILDING IS: LOW RISE
 - TERRAIN TYPE: OPEN WIND LOADING FOR OPEN STRUCTURES TO ASCE 7
- 14. LATERAL LOADS RESISTING SYSTEM: TIMBER BRACE FRAMES
- 15. BUILDING MEETS CI.4.1.8.1 (2) OF 2020 NATIONAL BUILDING CODE SEISMIC: Sa(0.2) = 0.0835
 - Sa(0.5) = 0.0502Sa(1.0) = 0.0259Sa(2.0) = 0.0112PGA = 0.0366
 - SITE CLASSIFICATION = C le = 1.0Fs = 1.0, Rs = 1.5, Ts = 0.34
- 16. DEFLECTION LIMITS: ROOF & FLOORS, TOTAL LOAD FLOORS & ROOFS, LIVE LOAD ROOF, SNOW LOAD

LATERAL STOREY DRIFT

= SPAN / 180 = SPAN / 360 = SPAN / 240 = HEIGHT / 500 (WIND) = HEIGHT x 0.02 (SEISMIC)

STEEL SCREW PILE NOTES

- 1. THE DESIGN, FABRICATION, AND INSTALLATION OF PILES SHALL BE
- GOVERNED BY THE FOLLOWING CODES: CSA G40.21 STRUCTURAL QUALITY STEELS
- CSA W48.14 MILD STEEL COVERED ARC-WELDING ELECTRODES CSA W48.5 MILD STEEL ELECTRONICS FOR FLUC-CORE ARC-WELDING
- CSA W47.1-09 CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL STRUCTURES
- PROVINCIAL OCCUPATIONAL HEALTH AND SAFETY REGULATIONS ASTM A252 WELDED AND SEAMLESS STEEL PIPE PILES
- ASTM A53 WELDED AND SEAMLESS PIPE
- 2. PILES SHALL BE DESIGNED FOR FACTORED LOAD SHOWN ON PLAN.
- 3. PILE WALL THICKNESS AND SHALL BE INCREASED AS NECESSARY TO ACCOMMODATE INSTALLATION EQUIPMENT.
- 4. SCREW PILES SHALL BE PLACED NOT CLOSER THAN 3 HELIX DIAMETERS FROM ADJACENT PILE U.N.O. PILES SHALL BE PLACED WITHIN 38mm (1 1/2") OF DESIGN DRAWINGS AND SHALL BE PLUMB TO 38mm (1 1/2"). SCREW PILES NOT WITHIN TOLERANCE MAY REQUIRE LOAD TESTING AT STRUCTURAL ENGINEERS DISCRETION.
- 5. SPACING OF HELICES SHALL NOT BE LESS THAN 3 TIMES THE HELIX
- 6. SPACING OF HELICES SHALL BE A WHOLE NUMBER MULTIPLE OF THE HELIX PITCH.
- 7. SUBMIT SCREW PILE LAYOUT AND SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF JURISDICTION.
- 8. SCREW PILES & PILE CAPS TO BE BARE STEEL.

MATERIAL SPECIFICATIONS

WOOD: TO CONFORM WITH CSA/CAN 086.19 ENGINEERING DESIGN IN WOOD

- GRADES AND TYPES TO BE AS FOLLOWS U.N.O.:
- WOOD BASED SHEATHING: ORIENTED STRANDBOARD TO CAN-0473.0-93 GRADE 0-2. PLYWOOD TO CSA 0325-07
- STRUCTURAL JOISTS & PLANKS: SPF #2 OR BETTER STRUCTURAL LIGHT FRAMING: SPF #2 OR BETTER
- WOOD POSTS & BUILT UP COLUMNS: SPF #2 OR BETTER
- TO MEET NLGA VISUAL GRADING RULES U.N.O.: SPECIES: COLUMNS & VERTICAL BRACING: S-P-F
 - LODGEPOLE PINE "PINUS CONTORTA" S-P-F BEAMS & HOR. BRACING: S-P-F GRADE: No.1 OR BETTER
- 2. FINISH SIZE: NOMINAL DIMENSIONS (i.e. 8x10 = 7-1/2"x9-1/2") DRY SERVICE CONDITION (MC<19%)
- 3. APPLY RUBIO MONOCOAT EXTERIO HARDWAX OIL FINISH TO ALL LOGS/TIMBERS/GLT OR APPROVED EQUIVALENT (TINT/COLOUR TO BE APPROVED BY CLIENT).

- 1. TO CONFORM TO THE FOLLOWING U.N.O.: BOLTS WOOD TO WOOD & STEEL:
 ASTM A307
- BOLTS STEEL TO STEEL: ASTM A325
- THREADED ROD: F1554 Grade 36 DRIFT PINS: ASTM A307 OR CSA/CAN-G40.21 300W.

- 1. FABRICATE TO CAN/CSA S16.14.
- 2. PROVIDE STEEL TO CSA/CAN-G40.20/G40.21 OR ASTM STANDARD A992/A992M. THE FOLLOWING SHOULD HAVE GRADES MINIMUM OF:
- STRUCTURAL SHAPES HOLLOW STRUCTURAL SECTIONS 350W
- COLUMN BASE PLATES 300W MISCELLANEOUS PLATES
- 3. PIPE SECTIONS ASTM A53, 241W
- 4. HOT DIP GALVANIZE ALL HARDWARE. ALL FINISHES TO CLIENT OR ARCHITECTS SPECIFICATIONS.

CONCRETE REINFORCEMENT

1. PROVIDE REINFORCEMENT TO CAN/CSA-A23.3 AND CSA G30.18 AS FOLLOWS: 400 MPa FOR 15M OR LARGER

2. PROVIDE NEW DEFORMED BARS TO CSA G30.18 GRADE 400.

CAST-IN-PLACE CONCRETE

- 1. CONCRETE AS PER CSA A23.1 ALTERNATIVE 1 PERFORMANCE SPECIFICATION.
- 2. CONTRACTOR AND CONCRETE SUPPLIER TO ENSURE THAT MIX PROPERTIES MEET REQUIREMENTS FOR PLACING, FINISHING AND THE SPECIFIED PERFORMANCE.
- 3. CONCRETE SUPPLIER TO BE CERTIFIED BY THE ALBERTA READY MIXED CONCRETE ASSOCIATION OR SASKATCHEWAN READY MIXED CONCRETE ASSOCIATION.
- 4. PORTLAND CEMENT TYPE GU UNO OR REQUIRED BY EXPOSURE CLASS.
- 5. CONCRETE TO BE NORMAL DENSITY (MIN. 2300 kg/m³) UNLESS NOTED OTHERWISE. 6. NOMINAL MAXIMUM SIZE OF COARSE AGGREGATE TO BE 20 (3/4") UNLESS NOTED
- OTHERWISE. 7. CONCRETE MIX SCHEDULE:

ELEMENT	COMPRESSIVE STRENGTH (MPa) AT 28 DAYS	EXPOSURE CLASS	REMARKS
PILES CAPS	35	S2	

- 1. F-2 FOR ELEMENTS SUSCEPTIBLE TO FREEZING. N EXPOSURE CLASS FOR
- ELEMENTS NOT SUSCEPTIBLE TO FREEZING. 2. SPECIFIED CONCRETE STRENGTH AT 56 DAYS FOR CLASS S EXPOSURE
- 8. REFER TO CSA A23.1 FOR THE MAXIMUM WATER/CEMENT RATIO, MINIMUM COMPRESSIVE STRENGTH, AIR CONTENT, CURING REQUIREMENTS, CHLORIDE ION PENETRABILITY AND ALTERNATE CEMENT TYPES TO MEET THE REQUIREMENTS FOR
- 9. DO NOT ADD WATER TO CONCRETE ON SITE.

THE NOTED EXPOSURE CLASS.

- 10. VIBRATE ALL CONCRETE.
- 11. PROTECT CONCRETE FROM FREEZING. DO NOT PLACE CONCRETE AGAINST FROZEN GROUND. USE COLD WEATHER CONCRETING METHODS IN ACCORDANCE WITH CSA-A23.1 WHEN TEMP IS BELOW +5°C. MAINTAIN CONCRETE TEMPERATURE OF 10° IS MAINTAINED THROUGHOUT THE CURING PERIOD (MINIMUM 3 DAYS).
- 12. PROTECT CONCRETE FROM EXCESSIVE HEAT AND DRYING. USE HOT WEATHER CONCRETING METHODS IN ACCORDANCE WITH CSA-A23.1 WHENEVER THE OUTDOOR TEMPERATURE IS GREATER THAN 27°C.

13. TESTING

- 1. PROVIDE INSPECTION REPORTS PREPARED BY AN INDEPENDENT INSPECTION AND TESTING AGENCY AT THE COST OF THE CONTRACTOR.
- 2. ONE STANDARD TEST FOR EACH 75 CUBIC METERS OF CONCRETE, BUT NOT LESS THAN ONE TEST FOR CONCRETE CAST EACH DAY. THREE CONCRETE CYLINDERS FOR EACH TEST IN ACCORANCE WITH CSA A23.1 AND 2.

14. CONCRETE FORMWORK

- 1. FORMWORK TO CSA S269.1-16. FORMWORK DESIGN IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 2. COLUMN AND WALL FORMS SHALL REMAIN UNTIL CONCRETE HAS REACHED 15MPa OR MIN 1 DAY.

TIMBER FRAMING

- 1. CONFIRM ALL DIMENSIONS, CONNECTION GEOMETRY AND CONSTRUCTABILITY OF THE ASSEMBLIES BEFORE PROCEEDING WITH FABRICATION. REPORT DISCREPANCIES TO THE ENGINEER.
- 2. TIMBER FRAME SUPPLIER TO SUBMIT DIGITAL PDF COPY OF SHOP DRAWINGS FOR REVIEW AND APPROVAL OF THE ENGINEER PRIOR TO PROCEEDING WITH THE FABRICATION.
- 3. ALL CONNECTIONS TO MEET THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND CSA 086-19 ENGINEERING DESIGN IN
- 4. TEMPORARY SUPPORT AND TEMPORARY BRACING OF THE TIMBER ELEMENTS DURING ERECTION AND CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 5. UNLESS NOTED THE TENONS ARE TO BE MINIMUM OF 2 1/2" THK.
- 6. HOUSINGS ARE TYPICALLY 1/16" DRAUGHT BORED.

SELF-TAPPING TIMBER FASTENERS

- 1. TIMBER FASTENERS ARE IDENTIFIED ON STRUCTURAL DRAWINGS IN THE FOLLOWING FORMAT:
- # [DIAMETER] X [LENGTH] / [THREAD LENGTH] [THREAD TYPE] [HEAD TYPE]

FT = FULLY THREADED PT = PARTIALLY THREADED

CSK = COUNTERSINK HEAD CYL = CYLINDER HEAD HEX = HEX HEAD WSH = WASHER/PAN HEAD

2 - 10Ø X 200/100 PT CSK

= TWO, 10MM DIAMETER X 200MM LONG SCREWS WITH 100MM THREAD; PARTIALLY THREADED WITH COUNTERSINK HEAD

2. THE FOLLOWING TIMBER SCREWS ARE ACCEPTABLE UNLESS A SPECIFIC MANUFACTURER IS NOTED ON DRAWINGS. APPROVED PRODUCTS NOTED BELOW ARE ACCEPTABLE BASED ON THEIR MOST CURRENT CCMC OR ETA.

2.1. PARTIALLY THREADED SELF TAPPING SCREWS MANUFACTURER FASTENER TYPE HECO TOPIX SFS INTEC SFS WFC-T SFS WFR-T SFS WFD-T SWG (WÜRTH) ASSY 3.0 SK ASSY 3.0 ECOFAST ASSY 3.0 KOMBI T-STAR

2.2. FULLY THREADED SELF TAPPING SCREWS MANUFACTURER **FASTENER TYPE** TOPIX CC

SFS INTEC SFS WT-T SFS WR-T SFS WB ASSY PLUS VG

3. SCREW TYPES SPECIFIED ON STRUCTURAL DRAWINGS SUPERSEDE THE INFORMATION ABOVE U.N.O.

T-STAR

- 4. HOLES IN STEEL PLATE TO MATCH THE SCREW TYPE USED.
- 5. FOR PARTIALLY THREADED SCREWS, EMBED FULL THREAD INTO SECONDARY MEMBER OR TO A DEPTH AS SPECIFIED ON DRAWINGS.
- 6. WOOD TO WOOD CONNECTIONS TO HAVE WASHER OR PAN-HEAD U.N.O.
- 7. WHERE PRE-DRILLING OF SCREWS IS RECOMMENDED BY THE SCREW MANUFACTURER, HOLE DIAMETER TO BE STRICTLY AS PER
- 8. DO NOT USE IMPACT DRILLS FOR SCREW INSTALLATION.

MANUFACTURER'S RECOMMENDATIONS.

- 9. DO NOT USE A HAMMER TO ENGAGE THE FASTENER WITH THE WOOD.
- 10. FULLY THREADED FASTENERS TO BE INSTALLED IN A ONE STEP PROCESS. DO NOT STOP THE DRILL DURING THE INSTALLATION.

11. METRIC CONVERSIONS: <u>DIAMETER</u> 8MM = 5/16" 10MM = 3/8"

> 25MM = 1"SCREW / THREAD LENGTH ROUND UP TO NEAREST STANDARD LENGTH FOR MANUFACTURER

12MM = 1/2"

ROUGH CARPENTRY

- . WOOD FRAMING TO CONFORM TO NLGA STANDARD GRADING RULES FOR CANADIAN LUMBER AND CSA 086-19 ENGINEERING DESIGN IN
- 2. THE ROOF SHEATHING AND SUPPORTING ROOF MEMBERS HAVE BEEN DESIGNED AS A DIAPHRAGM. DIAPHRAGM CONNECTION REQUIREMENTS FOR THE ROOF DECKING ARE: 2 1/2" x 3.33mm GUN NAILS @ 6" O/C AT SUPPORTED PANEL EDGE AND AT 12" O/C ALONG INTERMEDIATE FRAMING MEMBERS.
- 3. WIRE NAILS, SPIKES AND STAPLES TO CSA 19111-1974.

FOUNDATIONS

- 1. A GEOTECHNICAL INVESTIGATION HAS NOT BEEN COMPLETED FOR THE SITE. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR FOUNDATION CAPACITY. PILES SHALL BE DESIGNED FOR FACTORED LOAD SHOWN ON PLAN.
- 2. REFER TO THE GEOTECHNICAL REPORT FOR DETAILED INFORMATION ON GEOTECHNICAL CONDITIONS, FOUNDATION RECOMMENDATIONS, AND FOR ALL EARTHWORK INCLUDING EXCAVATION, BACKFILL AND SUBGRADE PREPARATION.
- 3. LOCARE ALL UNDERGROUND UTILITIES PRIOR TO STARTING ALL FOUNDATION WORK.
- 4. PILE TOLERANCES: LOCATION: 75 (3") CUT OFF ELEVATION: +50 (+2"), -100 (-4")



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PAIMFC ARBOUR

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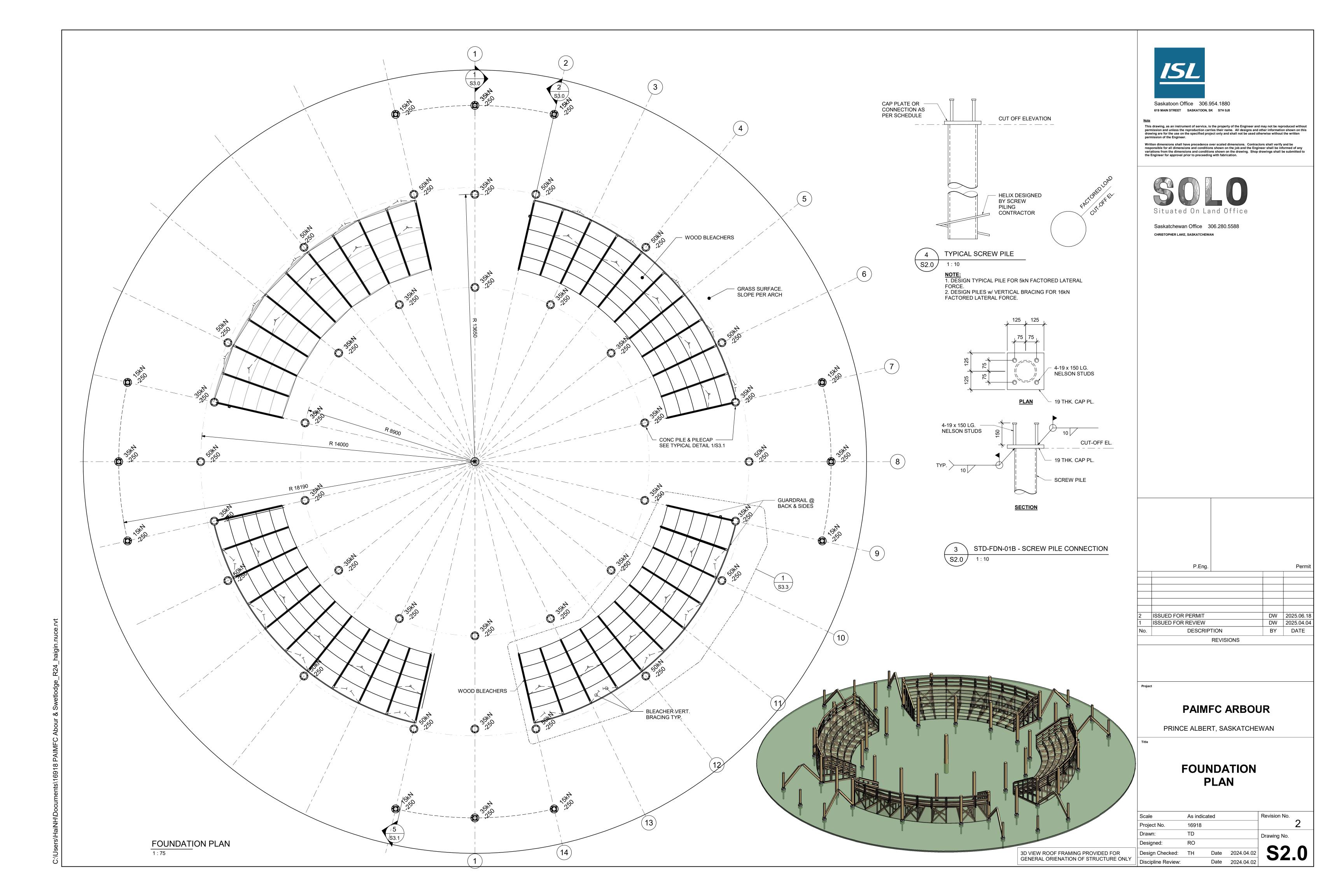
GENERAL NOTES

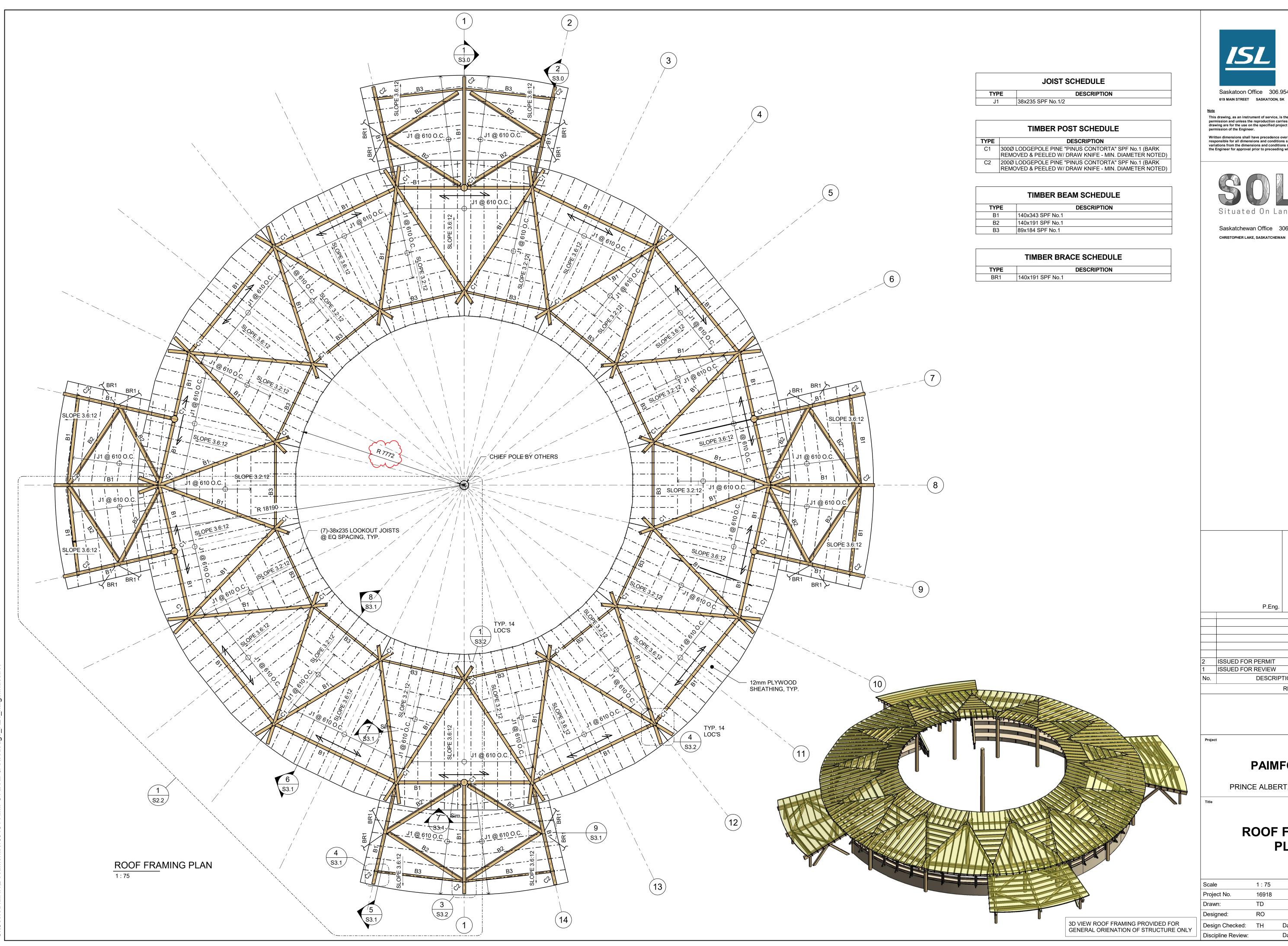
Date 2024.04.02

Drawing No.

Revision No. Scale 1:1 16918 Project No. Drawn: TD RO Designed: Date 2024.04.02 Design Checked: TH

Discipline Review:







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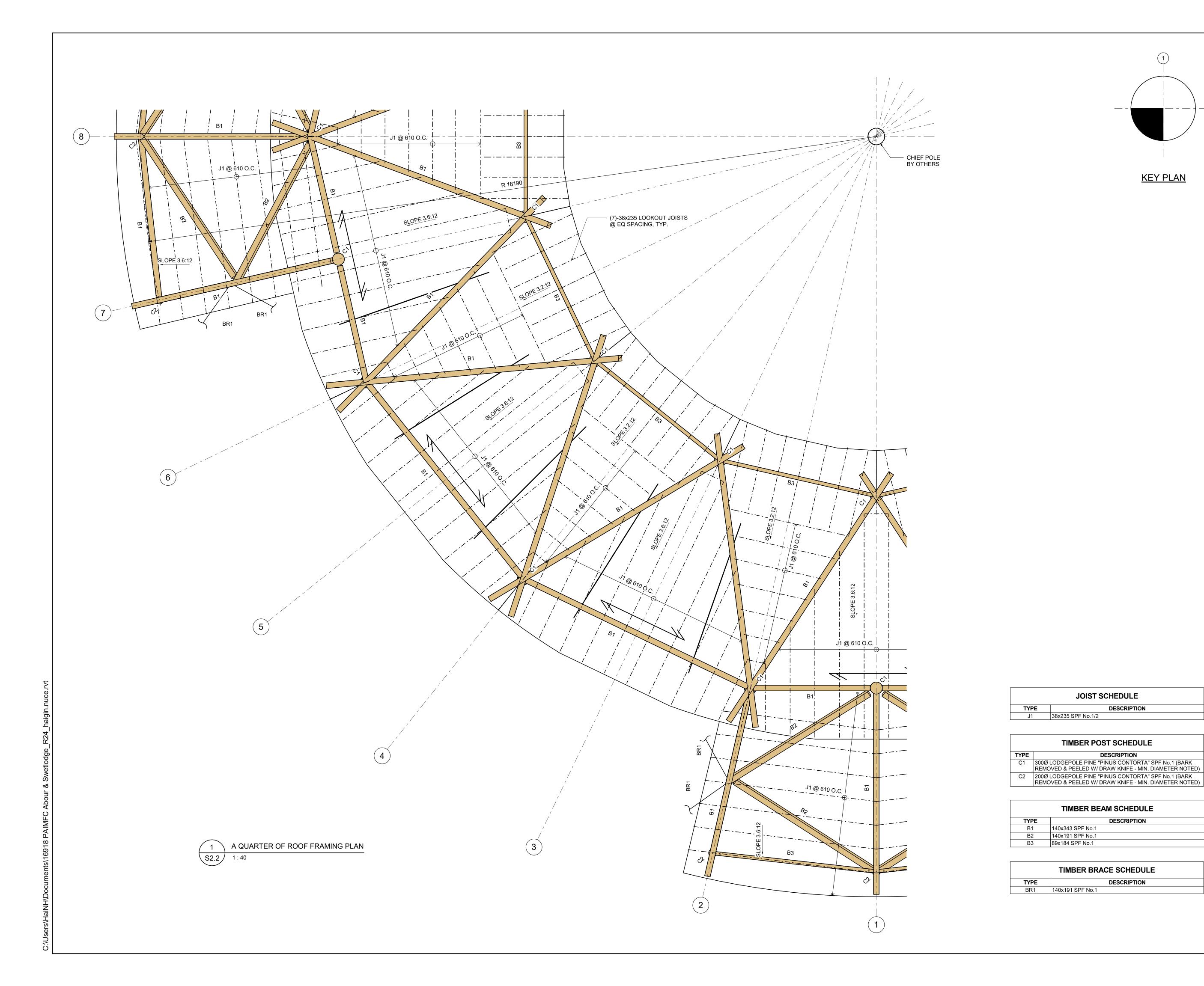
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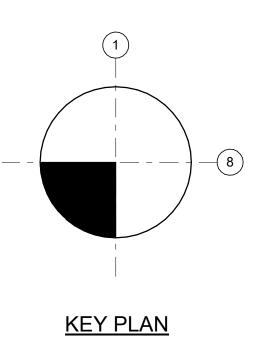
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ROOF FRAMING PLAN

Scale	1 : 75			Revision No.
Project No.	16918			
Drawn:	TD			Drawing No.
Designed:	RO			
Design Checked:	TH	Date	2024.04.02	S2
Discipline Review:		Date	2024.04.02	





JOIST SCHEDULE

TIMBER POST SCHEDULE

TIMBER BEAM SCHEDULE

DESCRIPTION

DESCRIPTION

DESCRIPTION

DESCRIPTION



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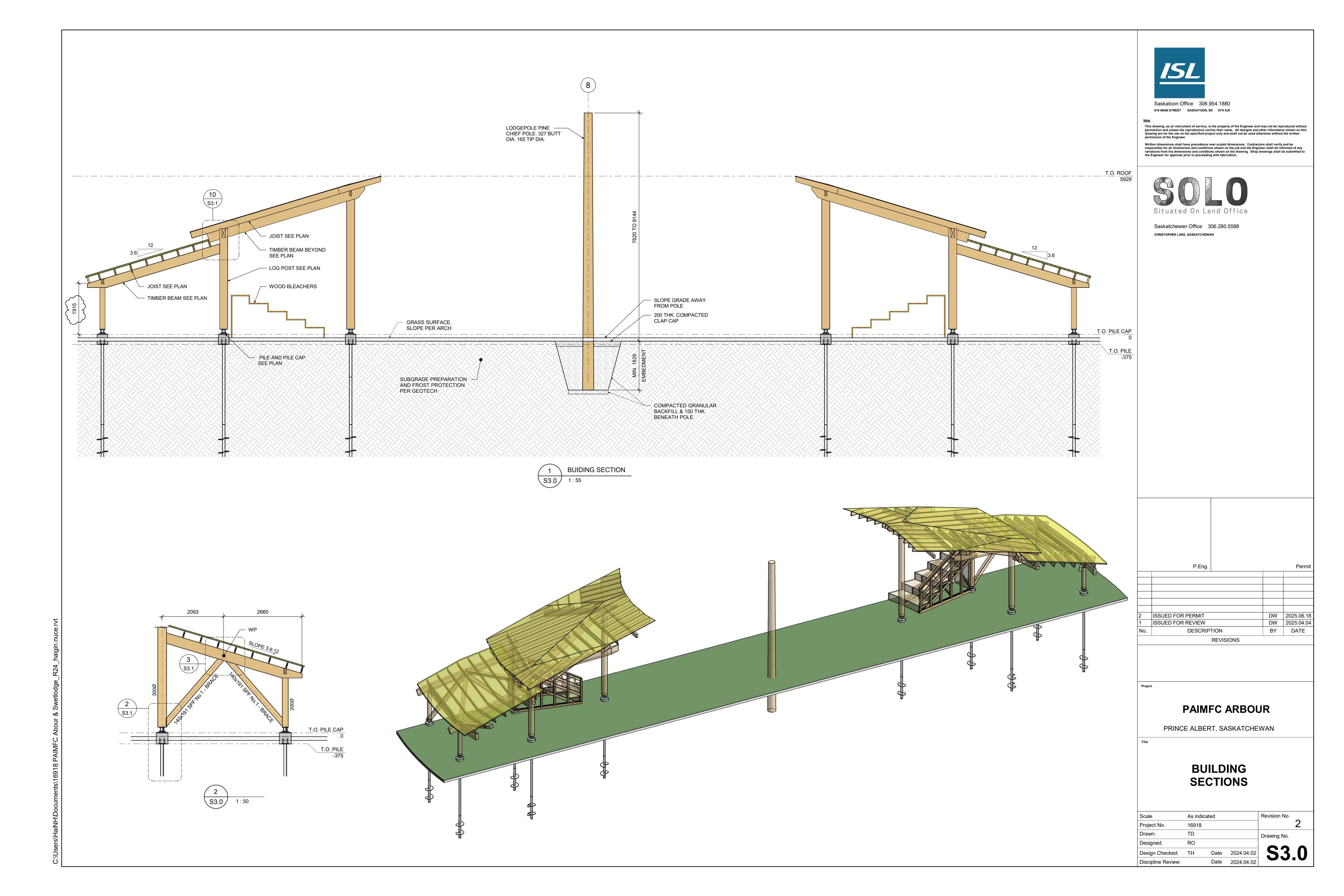
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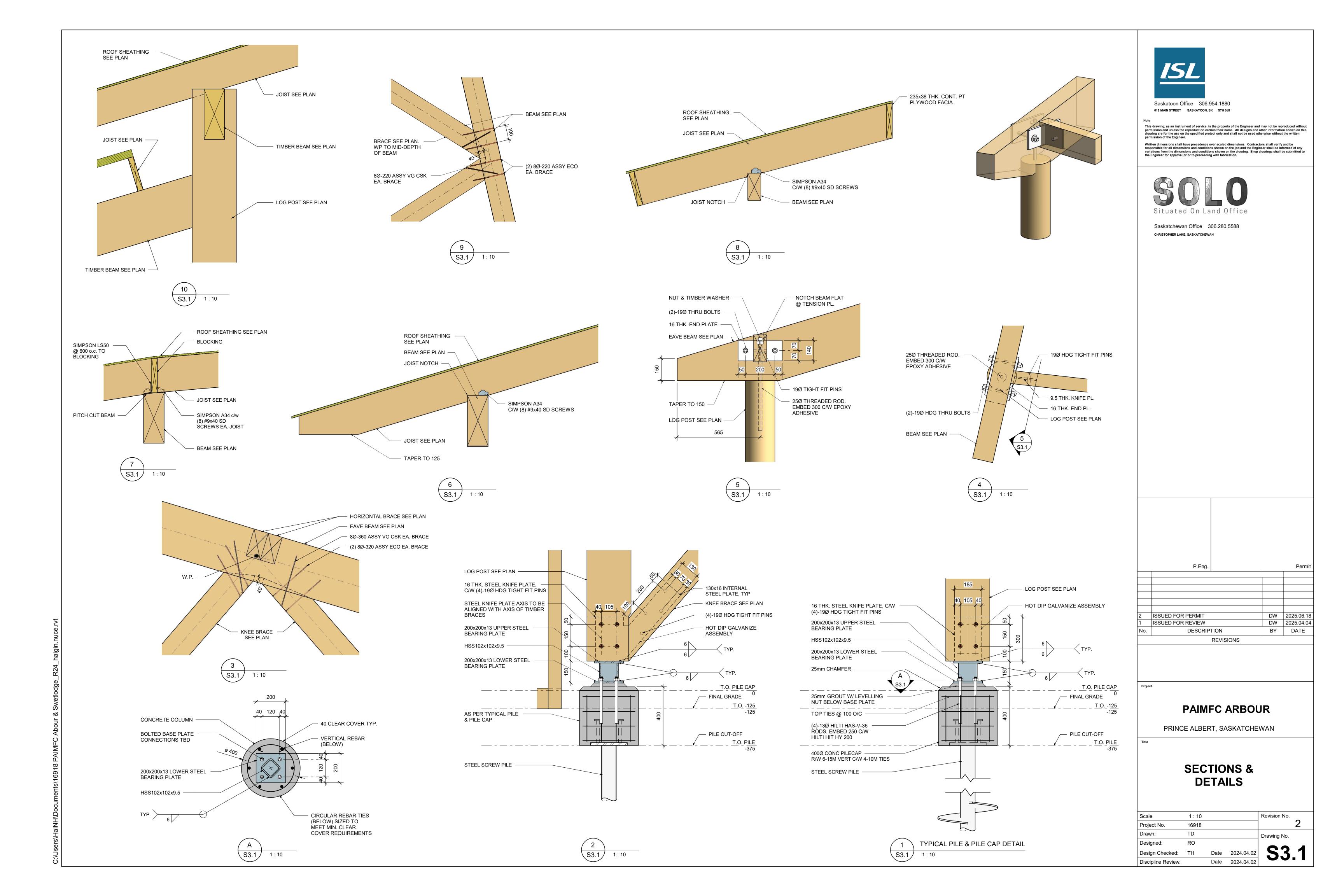
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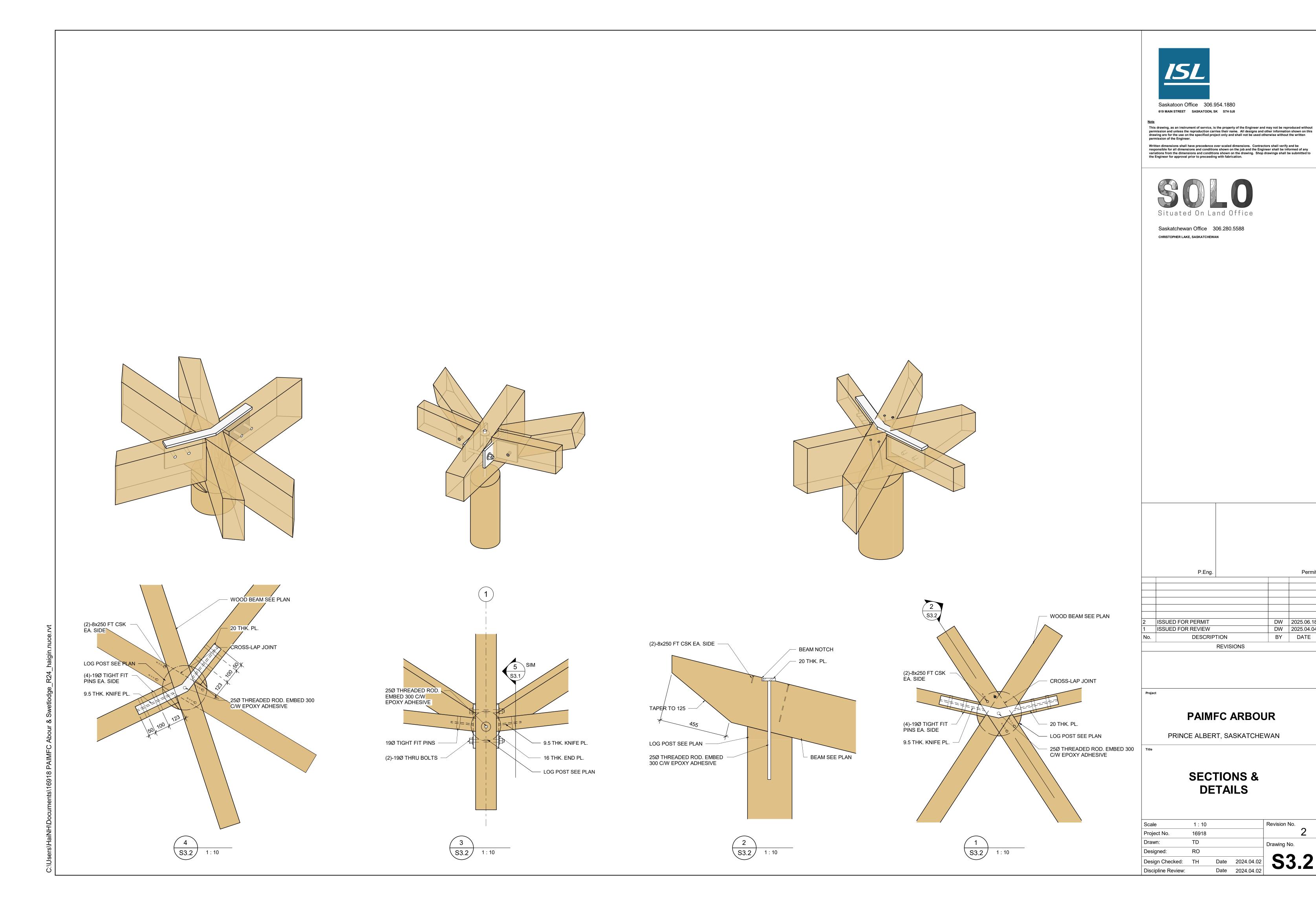
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A QUARTER OF **ROOF FRAMING PLAN**

Scale	As indica	ated		Revision No.
Project No.	16918			
Drawn:	TD			Drawing No.
Designed:	RO			
Design Checked:	TH	Date	2024.04.02	S 2
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Permit

DW 2025.06.18 DW 2025.04.04

BY DATE

Revision No.

