













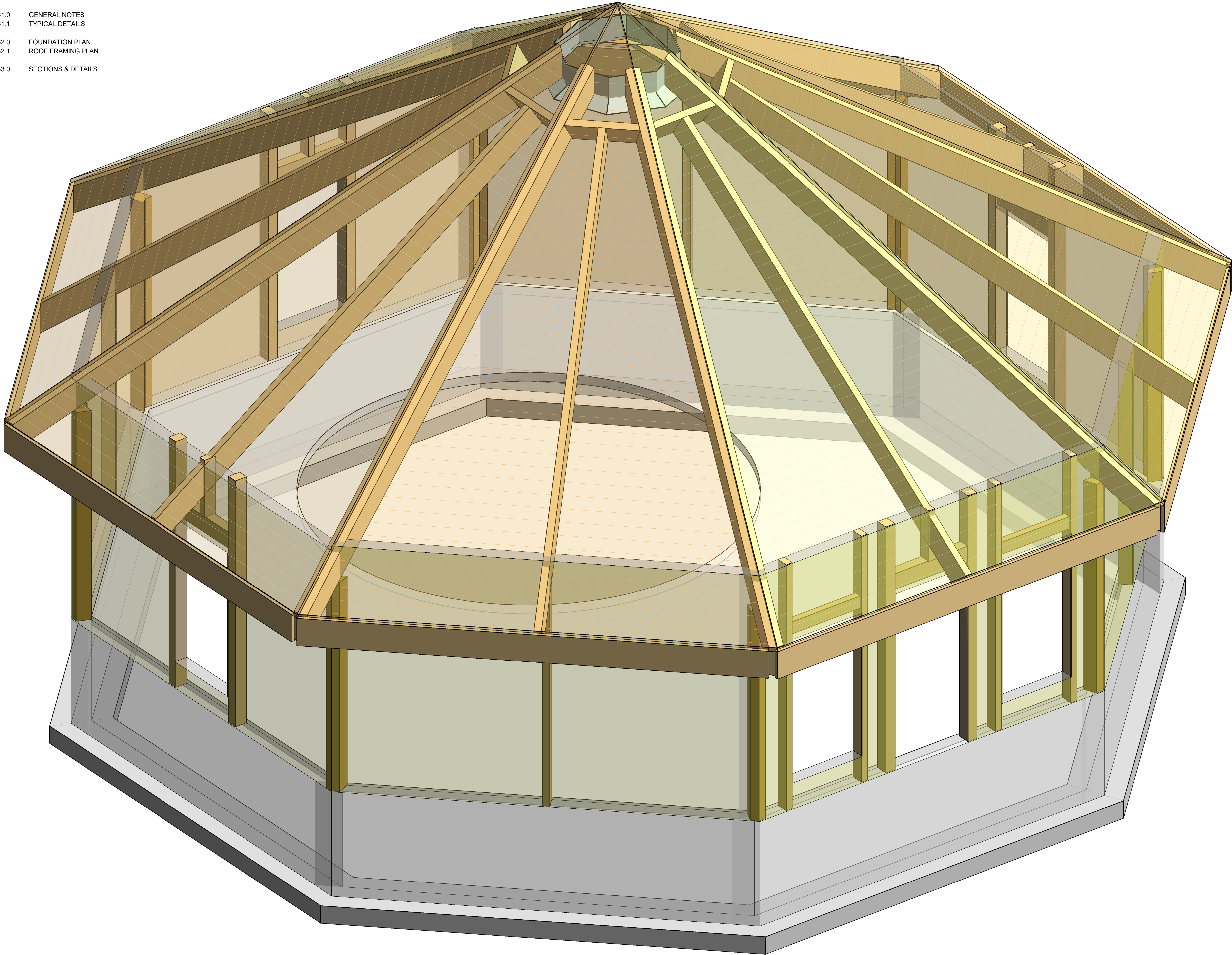
D:\03_REVIT\LOCAL FILE\16918 Sweatlodge_R24_vnbim.rvt

SWEATLODGE

PRINCE ALBERT, SASKATCHEWAN

DRAWING LIST

S0.0	COVER SHEET
S1.0	GENERAL NOTES
S1.1	TYPICAL DETAILS
S2.0	FOUNDATION PLAN
S2.1	ROOF FRAMING PLAN
S3.0	SECTIONS & DETAILS



Saskatoon Office 306.954.1880
619 MAIN STREET SASKATOON, SK S7N 0J8

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Association of Professional Engineers & Geoscientists
of Saskatchewan
CERTIFICATE OF AUTHORIZATION
ISL Engineering and Land Services Ltd.
Number C1096
ORIGINALLY STAMPED AND SIGNED
Discipline Str. Reg. No. Signature
STRUCT. 23928

Permit

2	ISSUED FOR PERMIT	DW	2025.06.11
1	ISSUED FOR REVIEW	DW	2025.04.09
No.	DESCRIPTION	BY	DATE

REVISIONS

Project
SWEATLODGE
PRINCE ALBERT, SASKATCHEWAN

Title
COVER SHEET

Scale	12" = 1'-0"	Revision No.	2
Project No.	16918	Drawing No.	S0.0
Drawn:	TD		
Designed:	DW		
Design Checked:	TH	Date	Issue Date
Discipline Review:		Date	Issue Date

1. THE CONTRACTOR SHALL EXAMINE ALL CONTRACT DOCUMENTS, CHECK DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER FOR CLARIFICATION PRIOR TO COMMENCING CONSTRUCTION. DIMENSIONS NOT REPORTED ARE THE RESPONSIBILITY OF THE CONTRACTOR. CHECK AND VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING WITH ANY WORK. NOTIFY THE ARCHITECT OF ANY ERRORS OR OMISSIONS.
2. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS
3. TEMPORARY SUPPORT AND TEMPORARY AND PERMANENT BRACING OF LOAD BEARING AND NON-LOAD BEARING ELEMENTS DURING CONSTRUCTION TO RESIST DEAD, LIVE AND CONSTRUCTION LOADS IS THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN OF THE TEMPORARY SUPPORTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
4. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
5. THE GENERAL CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR REVIEW BY THE ENGINEER OF RECORD. ALLOW ADEQUATE TIME FOR REVIEW BY THE ENGINEER OF RECORD PRIOR TO FABRICATION OR ERECTION. SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION FOR REVIEW BY THE ENGINEER OF RECORD. SHOP DRAWINGS SHALL BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT PROVINCE OR STATE OF JURISDICTION WHERE INDICATED. THE SHOP DRAWINGS SHALL REQUIRE THAT THE FABRICATION AND ERECTION OF THESE ELEMENTS ARE IN ACCORDANCE WITH THEIR DESIGN AND THAT THE DESIGN IS IN ACCORDANCE WITH ALL RELEVANT CODES AND REGULATIONS. SHOP DRAWINGS SHALL BE SUPPLIED FOR REVIEW FOR THE FOLLOWING BUT NOT LIMITED TO:

- * REINFORCING STEEL FOR CONCRETE
 - * ENGINEERED WOOD PRODUCTS & ROOF TRUSSES (P.ENG. SEALED)
 - * HEAVY TIMBER
6. ALL DESIGN TO CONFORM TO THE NBC2020 AND ALL OTHER APPLICABLE CODES AND BEST PRACTICES.
7. FIELD REVIEWS:
NOTIFY THE ENGINEER 48 HOURS IN ADVANCE FOR FIELD REVIEWS AND APPROVAL OF THE FOLLOWING:
- * CONCRETE REINFORCEMENT BEFORE EACH CONCRETE POUR
 - * WOOD FRAMING BEFORE COVERING UP
8. DO NOT SCALE FROM THESE DRAWINGS.
9. THE DESIGN HAS BEEN PREPARED BASED ON THE ASSUMPTION THAT THE OWNER AND/OR OPERATOR HAS A SITE SAFETY PLAN IN PLACE TO ADDRESS AND MITIGATE SAFETY HAZARDS, BOTH COMMON AND SPECIFIC TO THIS PROJECT

AFB	ASPHALT IMPREGNATED	J.D.	JOINT
	FIBRE BOARD	J.L.	LONG
ALT.	ALTERNATE	L.L.	LONG LEAD
ARCH.	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
B.C.E.	BOTTOM CHORD	LLV	LONG LEG VERTICAL
	EXTENSION	LSH	LONG SIDE HORIZONTAL
BL	BOTTOM LOWER LAYER	LSV	LONG SIDE VERTICAL
BUL	BOTTOM UPPER LAYER	L.V.	LENGTH VARIES
BUM	BEAM	L.W.	LONG WAY
BOT.	BOTTOM	MAX.	MAXIMUM
BR.	BAR PLACING ORDER	MECH.	MECHANICAL
BTWN.	BETWEEN	MIN.	MINIMUM
BRG.	BEARING	N.I.C.	NOT IN CONTRACT
CANT.	CANTILEVER	N.S.	N.EAR SIDE
C.J.	CONTROL JOINT	N.T.S.	NOT TO SCALE
CL.	CENTER LINE	O.C.	ON CENTRE
CL.	CLEAR	O.F.	OUTSIDE FACE
COL.	COLUMN	OPP.	OPPOSITE
CONC.	CONCRETE	OWSJ	OPEN WEB STEEL JOIST
CONT.	CONTINUOUS	PLA	POINT LOAD ABOVE
C.P.	COMPLETE	P/T	PRESSURE TREATED
	PENETRATION	R/D	ROOF DRAIN
CW	COMPLETE WITH	REIN.	REINFORCING
DET	DETAIL	R/W	REINFORCED WITH
D.L.	DEAD LOAD	S.D.L.	SUPERIMPOSED DEAD
D.O.	DO OVER (DITTO)		LOAD
DP.	DEEP	SIM.	SIMILAR
DWG.	DRAWING	S.O.G.	SLAB ON GRADE
DWLS	DOWELS	STAG.	STAGGERED
E.	EACH END	STIR.	STIRRUP
E.E.	EACH FACE	S.W.	SHORT WAY
ELEC.	ELECTRICAL	TEMP.	TEMPERATURE
EL.	ELEVATION		REINFORCING
ELEV.	ELEVATION	THK.	THICK
E.S.	EACH SIDE	THRU	THROUGH
E.W.	EACH WAY	T.J.	TIE JOIST
EXIST.	EXISTING	T.L.	TOP LOWER LAYER
EX	EXTRA	T.O.	TOP OF
EXT.	EXTERIOR	T.O.C.	TOP OF CONCRETE
F.D.	FLOOR DRAIN	T.O.S.	TOP OF STEEL/SLAB
F.S.	FAR SIDE	TUL	TOP OF UPPER LAYER
FTG	FOOTING	TYP.	TYPICAL
G.V.	GALVANIZED	T & B	TOP AND BOTTOM
G.L.	GRID LINE	T & G	TONGUE AND GROOVE
H.1.E.	HOOK ONE END	U.N.O.	UNLESS NOTED
H.2.E.	HOOK 2 ENDS		OTHERWISE
H.D.G.	HOT DIPPED GALVANIZED	U/S	UNDERSIDE
HOR.	HORIZONTAL	VERT.	VERTICAL
HORIZ.	HORIZONTAL	WT.	WALL THICKNESS
I.F.	INSIDE FACE	W.P.	WORK POINT
INT.	INTERIOR		
PLA	POINT LOAD ABOVE		

DESIGN CODE : NBC 2020, PART 9 w/ PART 4 COMPONENTS

IMPORTANCE CATEGORY = NORMAL

ROOF

DEAD LOADS:

ROOF ASSEMBLY DEAD LOAD 0.60kPa (SUPERIMPOSED)

ENVIRONMENTAL LOADS PER NBC 2020:

DESIGN SNOW LOAD (Ss) 2.10 kPa

RAIN LOAD (Sr) 0.20 kPa

DESIGN SNOW LOAD 1.36 kPa (PART 9)

LIVE LOADS:

RESIDENTIAL 1.90 kPa

SOIL CONDITIONS:

THE FOUNDATION HAS BEEN DESIGNED BASED ON THE FOLLOWING ASSUMED VALUES:

SERVICE BEARING CAPACITY (SLS) 75 kPa

FACTORED ULTIMATE BEARING CAPACITY (ULS) 100 kPa

DESIGN DEFLECTION LIMITS:

1. WOOD FRAMING TO CONFORM TO NLGA STANDARD GRADING RULES FOR CANADIAN LUMBER AND CSA 086-19 ENGINEERING DESIGN IN WOOD (LIMIT STATES DESIGN)
2. TOE NAIL T&G ROOF DECKING w/ (1) HOT DIP GALVANIZED 3 1/2" COMMON NAIL PER SUPPORT.
3. THE WALL SHEATHING AND SUPPORTING MEMBERS HAVE BEEN DESIGNED AS SHEAR WALLS. SHEAR WALL CONNECTION REQUIREMENTS FOR THE WALL SHEATHING ARE: 2 1/2" LONG COMMON NAILS @ 6" O/C AT SUPPORTED PANEL EDGE AND AT 12" O/C ALONG INTERMEDIATE FRAMING MEMBERS.
4. WIRE NAILS, SPIKES AND STAPLES TO CSA 19111-1974.
5. ALL BUILT UP COLUMNS AT BEAM BEARING AND GIRDER TRUSS BEARING MUST BE CONTINUOUS FROM POINT OF BEARING THROUGH TO CONCRETE
6. BUILT UP COLUMNS MAY ONLY BE CALLED UP ON PLAN WHERE BEARING OCCURS - BUT ARE CONTINUOUS POINT OF BEARING THROUGH TO CONCRETE
7. ALL FRAMED EXTERIOR WALLS TO BE 2x6 @ 16"(400mm) O/C PER ARCH. U.N.O. ON PLANS.
8. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS. NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO COMMENCING WORK.

1. THE OWNER SHALL OBTAIN THE SERVICES OF A QUALIFIED TESTING AGENCY TO PERFORM COMPACTION TESTS AS REQUESTED BY THE ENGINEER.
2. EXCAVATE TO THE LEVELS NOTED ON THE DRAWINGS FOR THE EXTENT OF THE STRUCTURE. STRIP THE OVER-EXCAVATED AREA OF ALL SILT.
3. FILL TO THE DESIGN SUBGRADE WITH PITRUN GRAVEL COMPACTED TO 98% STANDARD PROCTOR DENSITY.
4. KEEP EXCAVATION FREE OF WATER WHILE FILL AND CONCRETE FOUNDATION IS PLACED.
5. PROTECT BOTTOM OF EXCAVATION FROM FROST. DO NOT PLACE CONCRETE ON FROZEN SOIL.
6. WHERE POSSIBLE, BACKFILL WALLS FROM BOTH SIDES SIMULTANEOUSLY TO EQUALIZE SOIL PRESSURE. WALLS RESISTING LATERAL EARTH PRESSURES ARE NOT TO BE BACKFILLED UNTIL THE BASEMENT AND MAIN FLOOR SLABS ARE IN PLACE.
7. REMOVE ALL VEGETATION, ORGANIC SOIL AND CONSTRUCTION DEBRIS FROM BUILDING AND CONSTRUCTION AREA TO EXPOSE INORGANIC SUBGRADE SOIL. PROVIDE A GOOD BASE FOR COMPACTING THE FIRST LIFT OF MATERIAL TO THE SPECIFIED DENSITY. ANY SOFT SUBGRADE SOIL ENCOUNTERED SHOULD BE SUB-EXCAVATED AND REPLACED WITH FREE DRAINING PITRUN GRAVEL. SOFT SUBGRADE SOIL WILL LIKELY BE ENCOUNTERED DURING SITE PREPARATION. COMPACT FREE DRAINING PITRUN GRAVEL TO NOT LESS THAN 98% STANDARD PROCTOR DRY DENSITY.
8. A MINIMUM OF 200mm THICK NON PLASTIC CRUSHED GRAVEL MUST BE PLACED BENEATH THE ENTIRE SLAB AND ABOVE THE PREPARED SUBGRADE SOIL. THE CRUSHED GRAVEL MUST BE UNIFORMLY COMPACTED TO 98% STANDARD PROCTOR DRY DENSITY. THE GRAVEL SHALL MEET THE FOLLOWING GRADATION STANDARD:

SIEVE SIZE	% PASSING BY WEIGHT
19mm	100
12.5mm	70-100
4.75mm	40-60
1.18mm	25-45
0.30mm	10-25
0.075mm	2-12
9. THE SLAB BASE GRAVEL AND SUBGRADE SOIL MUST BE PROTECTED FROM RAIN, SNOW, EXCESSIVE DRYING AND INGRESS OF FREE WATER DURING AND AFTER THE CONSTRUCTION TO PREVENT ANY FOUNDATION MOVEMENT.
10. REFER TO CIVIL DRAWINGS FOR GROUND ELEVATIONS AND DRAINAGE SLOPES.
11. CONFIRM EXACT LOCATIONS OF ALL UTILITY LINES WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE COMMENCEMENT OF EXCAVATION.
12. REFER TO GEOTECHNICAL ENGINEERS RECOMMENDATIONS AND SUPERCEDE THESE IF SPECIFIED

WOOD: TO CONFORM WITH CSA/CAN 086. 19 ENGINEERING DESIGN IN WOOD (LIMIT STATES DESIGN)

FRAMING GRADES AND TYPES TO BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

- 1. WOOD BASED SHEATHING:**
ORIENTED STRANDBOARD TO CAN-0473.0-93 GRADE 0-2.
PLYWOOD TO CSA 0325-07
- 2. STRUCTURAL JOISTS & PLANKS:**
SPF #2 OR BETTER.
- 3. STRUCTURAL LIGHT FRAMING:**
SPF #2 OR BETTER.
- 4. WOOD POSTS & BUILT UP COLUMNS:**
SPF #2 OR BETTER.

TIMBER

TO MEET NLGA VISUAL GRADING RULES, UNLESS NOTED OTHERWISE

SPECIES:	HEM-FIR
GRADE:	No.1 OR BETTER

- 2. FINISH SIZE:** FULL SIZE ROUGH SAWN TIMBERS AS NOTED
MOISTURE CONTENT : <19% AT TIME OF FABRICATION
- 3. CONFIRM ALL DIMENSIONS, CONNECTION GEOMETRY AND CONSTRUCTABILITY OF THE ASSEMBLIES BEFORE PROCEEDING WITH FABRICATION. REPORT DISCREPANCIES TO THE ENGINEER**
- 4. ALL CONNECTIONS TO MEET THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND CSA 086-19 ENGINEERING DESIGN IN WOOD**
- 5. TEMPORARY SUPPORT AND TEMPORARY BRACING OF THE TIMBER ELEMENTS DURING ERECTION AND CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR**

1. TO CONFORM TO THE FOLLOWING U.N.O.:

• BOLTS WOOD TO WOOD & STEEL :	ASTM A307
• BOLTS STEEL TO STEEL :	ASTM A325
• THREADED ROD :	F1554 Grade 36

1. CAST-IN-PLACE CONCRETE AND CONSTITUENT MATERIAL SHALL COMPLY WITH CSA A23.1
2. PRECAST CONCRETE AND CONSTITUENT MATERIAL SHALL COMPLY WITH CSA A23.4, EXCEPT AS SPECIFIED BY THE ENGINEER.
3. CONCRETE SHALL BE PROPORTIONED AND PRODUCED IN ACCORDANCE WITH CSA A23.1 OR CSA A23.4.
4. CONCRETE SHALL BE MIXED, PLACED AND CURED IN ACCORDANCE WITH CSA A23.1 OR CSA A23.4.
5. CONCRETE IS TO BE PLACED TO CSA A23.1 1-M90 AS FOLLOWS:

MEMBER	MINIMUM 28-DAY STRENGTH (MPa)	EXPOSURE CLASS
INTERIOR SLABS ON GRADE	32	R-3
FOOTINGS & FOUNDATION WALLS	25	R-1

6. MAXIMUM AGGREGATE SIZE TO BE 20mm.
7. DO NOT USED ADMIXTURES OTHER THAN AIR ENTRAINMENT, STANDARD WATER REDUCERS OR SUPER PLASTICIZERS WITHOUT PRIOR APPROVAL OF ENGINEER. CALCIUM CHLORIDE ADMIXTURE ARE NOT PERMITTED UNLESS NOTED.
8. PROVIDE 4-7% AIR ENTRAINMENT FOR ALL EXTERIOR CONCRETE.
9. REJECT ALL CONCRETE WHEN TIME BETWEEN BATCHING AND PLACING EXCEEDS 2 HOURS.
10. DO NOT ADD WATER TO CONCRETE ON SITE .

1. CONSTRUCT FORMWORK IN ACCORDANCE WITH WCB REGULATIONS AND CSA S269.3. FORMWORK DESIGN IS THE RESPONSIBILITY OF THE CONTRACTOR.
2. NO COLUMN OR WALL FORMS SHALL BE REMOVED BEFORE CONCRETE HAS REACHED 75% OF DESIGN STRENGTH OR 15MPa FOR ARCHITECTURAL CONCRETE.

1. TIE ALL BARS SECURELY IN PLACE TO PREVENT DISPLACEMENT.
SUPPORT SLAB REINFORCEMENT ON SUITABLE CHAIRS OR SUPPORTS AT MAXIMUM 1.2 METRE CENTRES. PROVIDE CORNER BARS TO MATCH HORIZONTAL WALL REINFORCEMENT.
2. CLEAR COVER TO REINFORCEMENT (PRINCIPAL REINFORCEMENT) IS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	75mm
EXPOSED TO EARTH OR WEATHER:	50mm
20M OR GREATER	50mm
15M OR SMALLER	40mm

NOT EXPOSED TO EARTH OR WEATHER OR NOT IN CONTACT WITH THE GROUND:

SLABS AND WALLS (35M OR SMALLER)	20mm
BEAMS AND COLUMNS	40mm
3. UNLESS OTHERWISE NOTED, USE CLASS B TENSION SPLICE FOR ALL REINFORCING STEEL.
4. UNLESS OTHERWISE NOTED, PROVIDE DOWELS TO MATCH VERTICAL REINFORCING WHEREVER A PILASTER, PIER OR WALL BEGINS.
5. SPLICE REINFORCEMENT AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWING:

BAR SIZE	10M	15M	20M	25M	30M
LAP SPLICE	450mm	600mm	750mm	1200mm	1450mm

INCREASE LAP 20% FOR BAR SPACING LESS THAN 6"

1. CONCRETE IS TO BE TESTED IN ACCORDANCE WITH CSA A23.1-14/A23.2-14 AND BY A QUALIFIED TESTING AGENCY AS REQUESTED BY THE ENGINEER. THREE TEST CYLINDERS SHOULD BE TAKEN FOR EVERY 15 CU METERS OR LESS OF CONCRETE PLACED. A MINIMUM OF ONE TEST OF THREE CYLINDERS IS REQUIRED PER POUR.
2. SAWCUT CONTROL JOINTS FOR SLABS ON GRADE AS SOON AS POSSIBLE SPACED AT 25 TIMES THE SLAB THICKNESS, NOT EXCEEDING 4.5m SPACINGS OR AT LOCATIONS SHOWN ON DRAWINGS, AS PER CSA A23.
3. ENSURE SLEEVES, TIES, ANCHOR BOLTS, PIPE HANGERS AND ANY OTHER INSERTS OR OPENINGS REQUIRED IN THE CONCRETE BY OTHER TRADES ARE COMPLETED.
4. ENSURE WELDED WIRE MESH CONFORMS TO CSA G30.5.
5. INSTALL ALL CONSTRUCTION JOINTS AS SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE ENGINEER.
6. COLD WEATHER CONCRETING
 - A. COLD WEATHER CONCRETING IS TO COMPLY WITH CSA/CAN- A23.1 AND ACI STANDARD 306R-10.
 - B. USE HOT WATER WHEN TEMPERATURE IS BELOW +3°C.
 - C. A MAXIMUM OF 12% CALCIUM CHLORIDE MAY BE USED EXCEPT FOR POST TENSIONED AND PARKING SLABS.
 - D. WHERE SUPPLEMENTARY HEAT IS PROVIDED, USE APPROVED CONCRETE HEATERS WITH EXHAUST VENTED AWAY FROM SURFACE OF CONCRETE.
 - E. FOR TEMPERATURES BELOW -10°C, PROVIDE CONCRETE POUR PROCEDURES TO ENGINEER FOR REVIEW
7. PROCEDURES (TEMPS SPECIFIED ARE MINIMUM TEMPS):

FOUNDATIONS
ABOVE 0°C: NO SPECIAL REQUIREMENTS
-3°C TO 0°C: COVER WITH INSULATION BLANKET FOR FIRST 24 HOURS
BELOW -3°C: DO NOT POUR ON FROZEN SOIL. COVER AND PROVIDE SUPPLEMENTARY HEAT FOR FIRST 24 HOURS. ALTERNATIVELY USE INSULATED BLANKET
- FLOOR SLAB ON GRADE AND SUSPENDED SLABS
0°C TO +3°C: COVER WITH POLY RAISED UP ON 2x4 SLEEPERS
COVER WITH INSULATION BLANKET FOR FIRST 36 HOURS
BELOW -3°C: DO NOT POUR ON FROZEN SOIL. COVER WITH BLANKET AND HEAT FOR FIRST 36 HOURS. FOR SUSPENDED SLABS, PROVIDE HEAT TO SPACE BENEATH

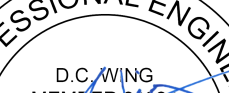
- HOT WEATHER CONCRETING (FOR SLABS ON GRADE, SUSPENDED SLABS & TILT UP CONCRETE PANELS)**
- A. REVIEW CONCRETE REQUIREMENTS WITH SUPPLIER PRIOR TO DAY OF POUR**
- B. DO NOT USE RETARDERS OR ACCELERATORS IN MIX UNLESS AUTHORIZED BY ENGINEER.**
- C. KEEP SUPERPLASTICIZER AVAILABLE AT SITE TO INCREASE WORKABILITY. DO NOT EXCEED RECOMMENDED STANDARD DOSAGES.**
- D. DO NOT ATTEMPT LARGE POURS ON HOT DAYS. SCHEDULE POURS EARLY IN THE MORNING FOR A NOON COMPLETION**
- E. FOR SLABS ON GRADE, DAMPEN THE SUBGRADE THE DAY BEFORE THE POUR.**
- F. PLACING POLY BENEATH SLABS IS SOMETIMES BENEFICIAL IN HOT WEATHER POURS. POLY SHOULD BE PERFORATED WITH DRAIN HOLES 300MM APART. RETARDERS UNLESS POLY IS REQUIRED AS A VAPOUR BARRIER. CHECK WITH ENGINEER**
- G. ERECT WIND BREAKS TO PREVENT RAPID AND EXCESSIVE MOISTURE LOSS.**
- H. PLACE CONCRETE DIRECTLY FROM TRUCK CHUTE WHERE POSSIBLE.**
- I. SURE THERE ARE SUFFICIENT PLACERS AND FINISHERS ON SITE**
- J. USE AN EVAPORATION RETARDANT SUCH AS 'CONFILM' BY MASTER BUILDERS. DO NOT ATTEMPT TO PLACE FLATWORK ON SUNNY DAYS WHERE PREDICTED TEMPERATURES EXCEED +30°C.**
- K. FOR CURING, APPLY SURFACE SEALERS AS SOON AS POSSIBLE AFTER THE FINAL TROWEL**
- L. WET CURE CONCRETE FOR AT LEAST 3 DAYS USING CONTINUOUS SPRINKLING OR FLOODING. DO NOT USE UNDER POLY SHEETING OR WITH WET BURLAP.**

1. PROVIDE NEW DEFORMED BARS TO CSA G30.18 GRADE 400.

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CERTIFICATE OF AUTHORIZATION
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ORIGINALLY STAMPED AND SIGNED

Discipline	St. Reg. No.	Signature
STRUCT.	23928	
_____	_____	_____

P.Eng.

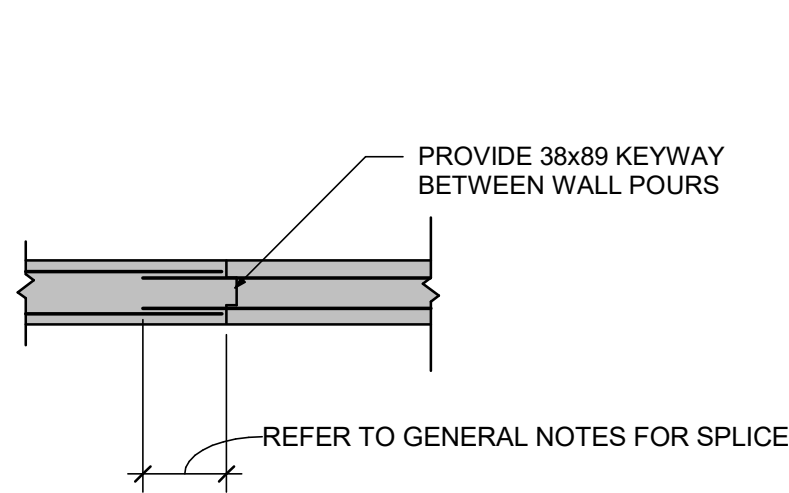
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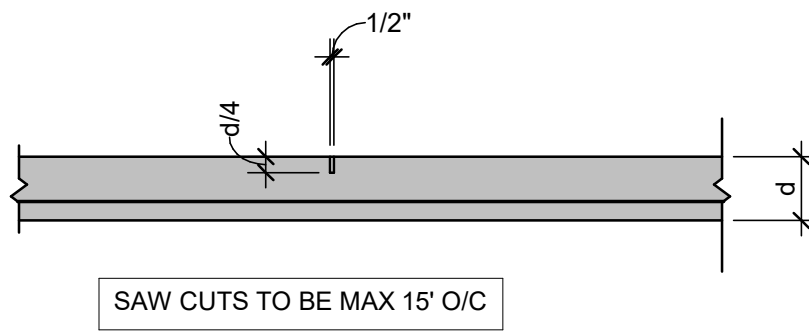
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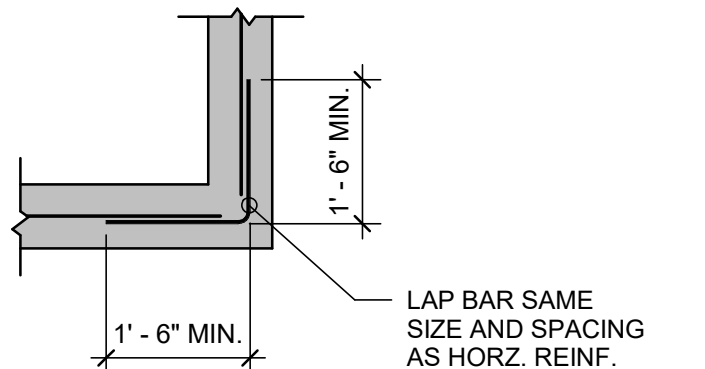
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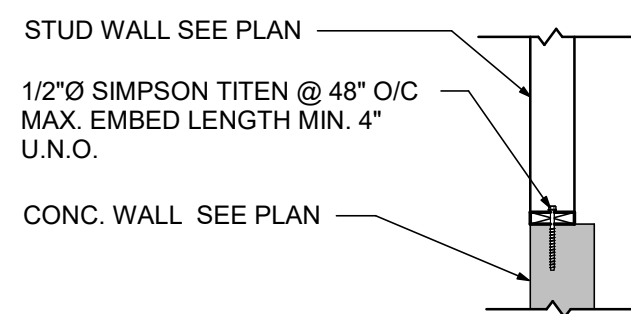
9 CONSTRUCTION JOINT IN CONC. WALL
S1.1 1/2" = 1'-0"



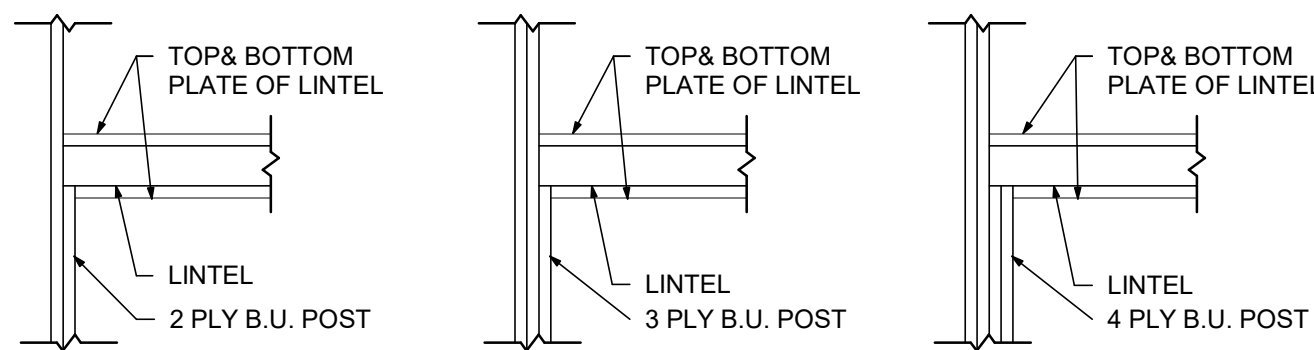
8 SAWCUT CONTROL JOINT
S1.1 1/2" = 1'-0"



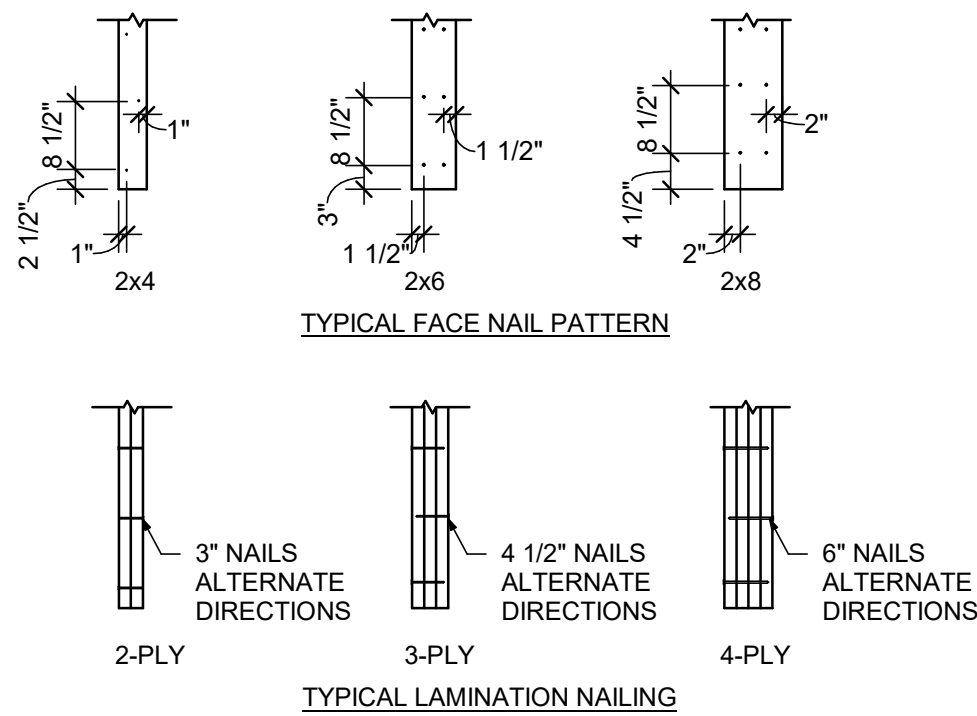
7 WALL CORNER DETAIL
S1.1 1/2" = 1'-0"



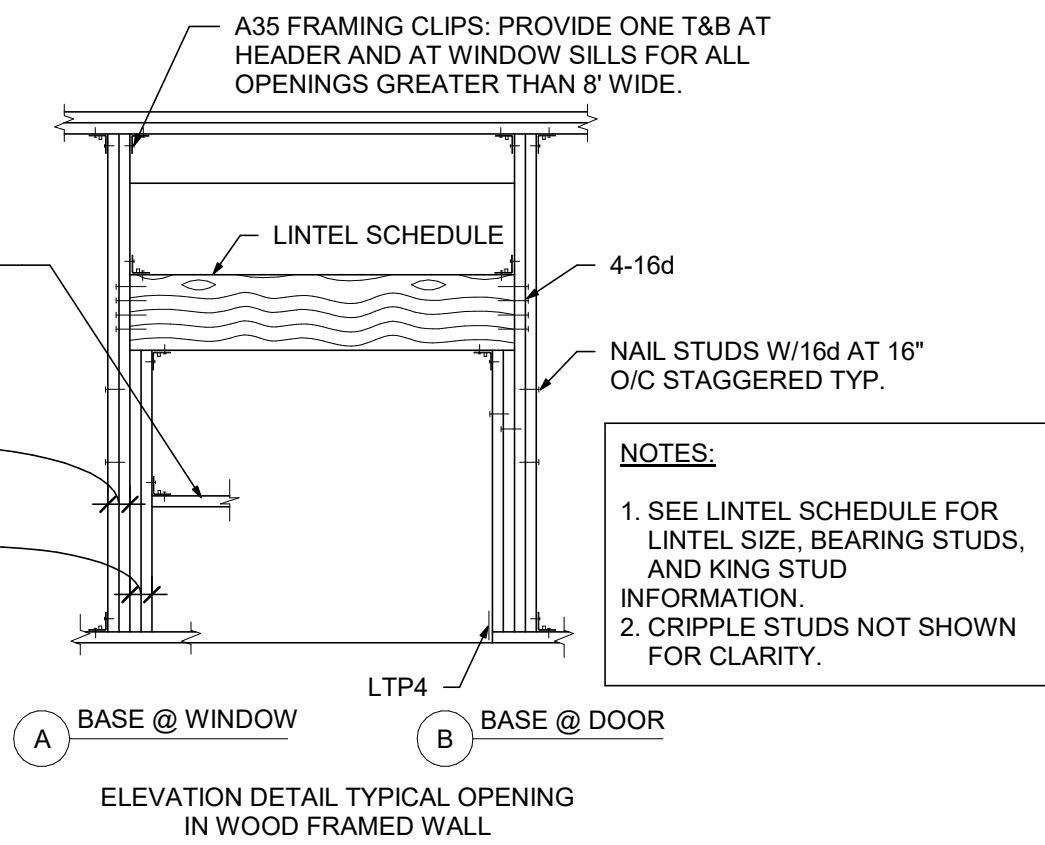
5 STUD WALL TO CONC. WALL
S1.1 1/2" = 1'-0"



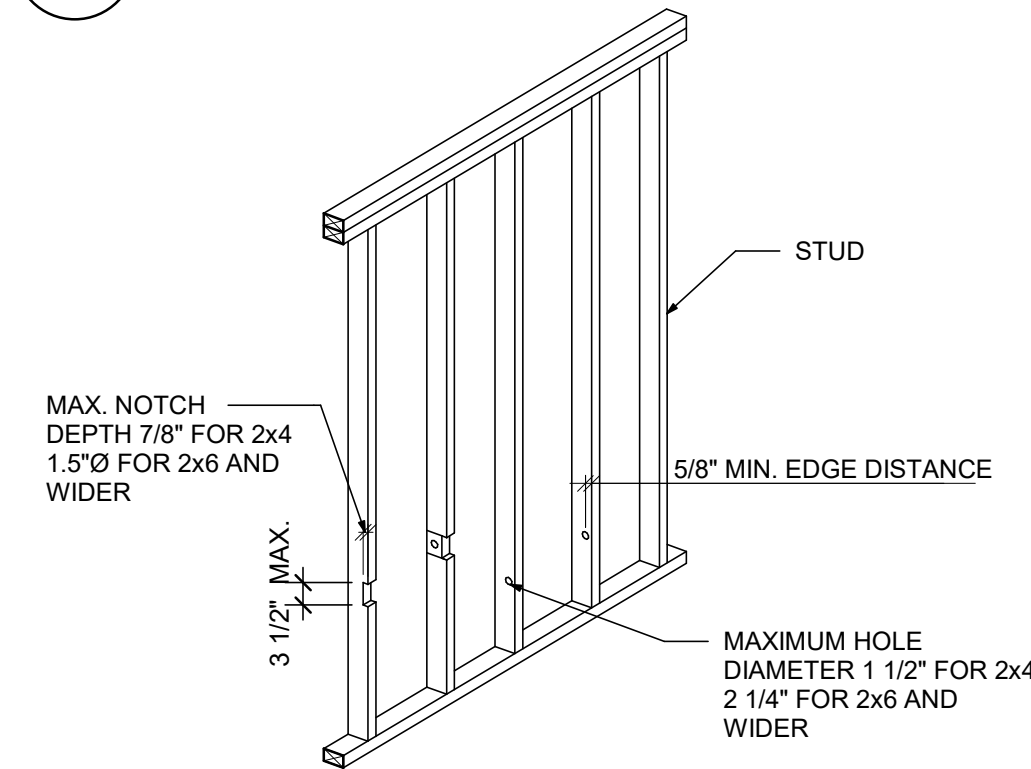
4 LINTEL TO POST CONNECTION
S1.1 1/2" = 1'-0"



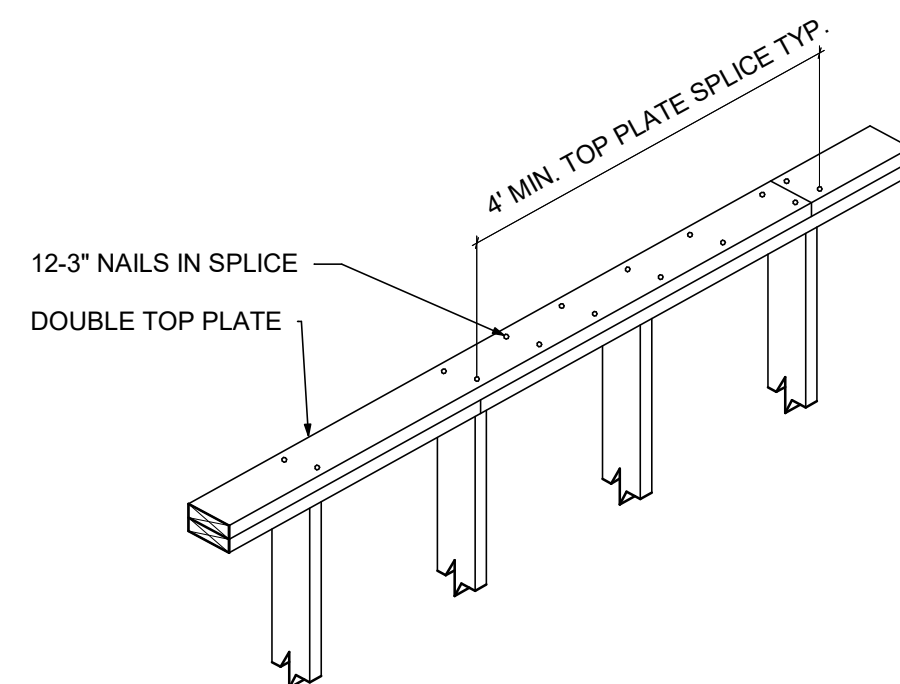
3 BUILT-UP POST NAILING DETAIL
S1.1 1/2" = 1'-0"



6 WINDOW FRAMING
S1.1 1/2" = 1'-0"



2 DRILLING AND NOTCHING
S1.1 1/2" = 1'-0"

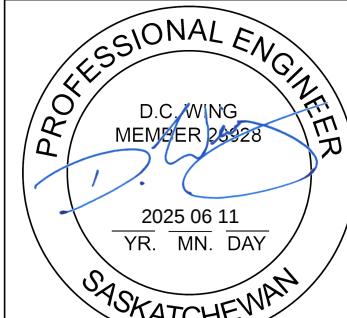


1 TOP PLATE SPLICE
S1.1 1/2" = 1'-0"



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Number C1096
2025.06.11
Discipline: STRUCT. 23928
Signature: [Signature]
P.Eng. Permit

No.	DESCRIPTION	BY	DATE
2	ISSUED FOR PERMIT	DW	2025.06.11
1	ISSUED FOR REVIEW	DW	2025.04.09

REVISIONS

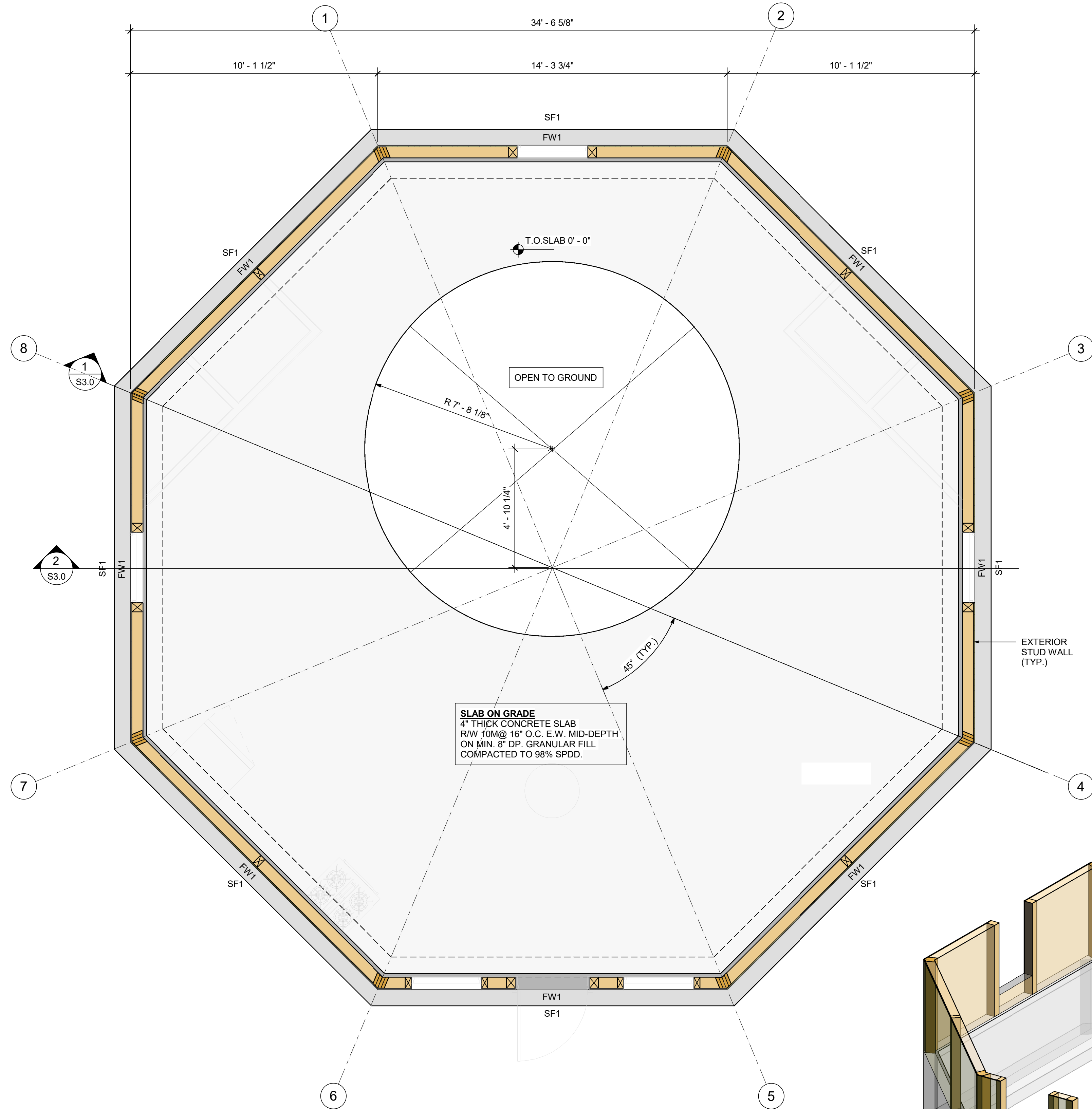
Project
SWEATLODGE
PRINCE ALBERT, SASKATCHEWAN

Title
TYPICAL DETAILS

Scale	1/2" = 1'-0"	Revision No.	2
Project No.	16918	Drawn:	TD
Design:	DW	Discipline Review:	TH
Design Checked:	TH	Date	Issue Date
Discipline Review:	Date	Issue Date	

S1.1

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FOUNDATION PLAN NOTES

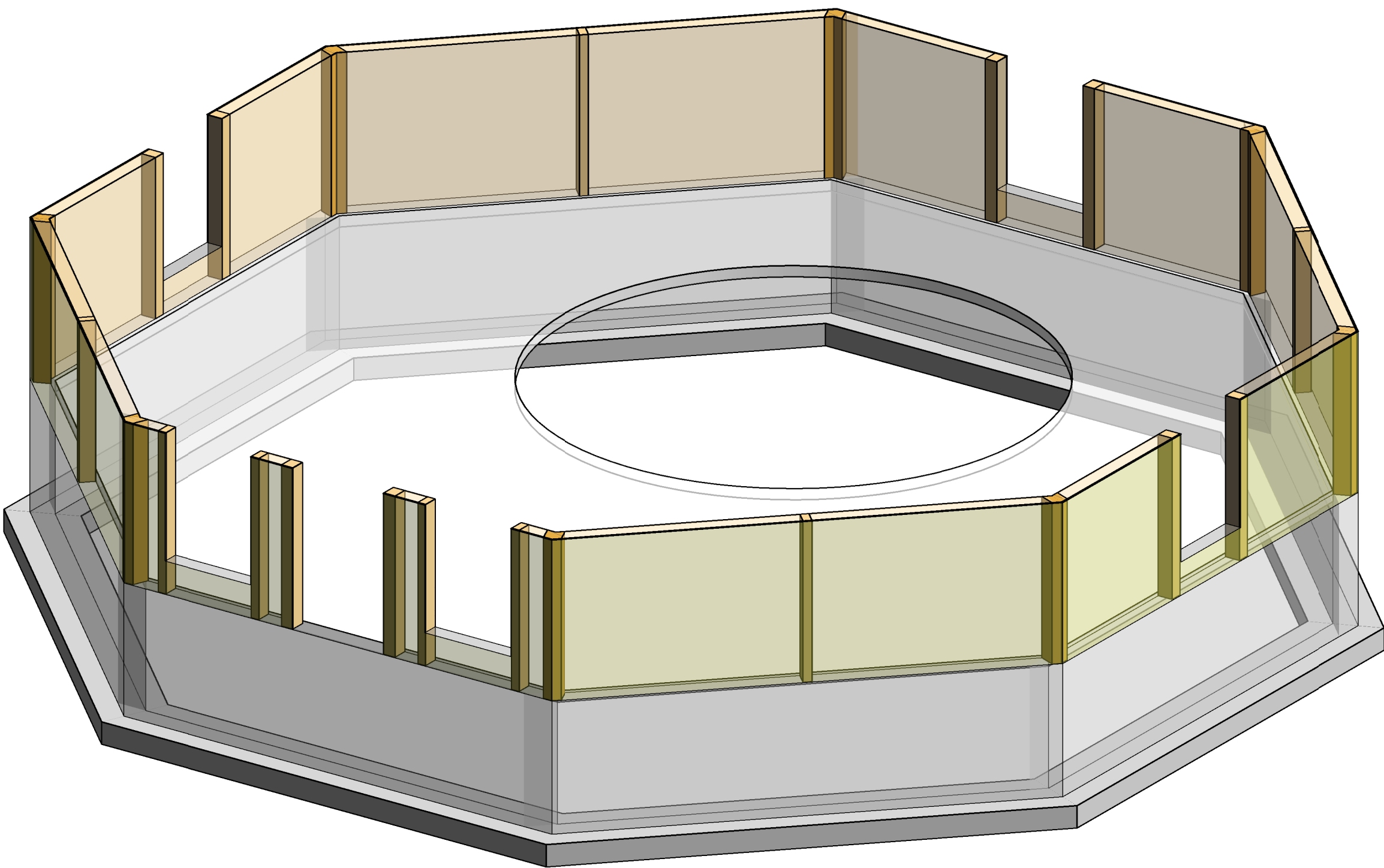
- SEE DWG. S1.0 & S1.1 FOR ADDITIONAL NOTES
- COORDINATE FLOOR DRAINS AND SLOPES WITH ARCH. DWGS.
- SEE MECH. DWGS FOR FOUNDATION DRAINAGE
- FOOTING LEVELS SHOWN ARE NOT FINAL AND MAY VARY ACCORDING TO SITE CONDITIONS. EXTEND ALL FOOTINGS TO A BEARING LAYER APPROVED BY THE GEOTECHNICAL ENGINEER.
- PROVIDE 15Mx12"x36" DOWELS FROM FOOTING TO WALL TO MATCH WALL VERT. REINF.
- CONC. PIERS & FOUNDATION WALL TO BE SEPARATED FROM SLAB W/ A.I.F.B. U.N.O.
- FOOTING ELEVATIONS ARE RELATIVE TO ADJACENT GRADE

FOUNDATION WALL SCHEDULE

TYPE	DESCRIPTION	VERT. REINF.	HORZ. REINF.	EXTRA
FW1	8" THK. CONC. WALL	15M@16" O/C C.L.	15M @ 16" O/C C.L.	2-15M ON TOP

STRIP FOOTING SCHEDULE

TYPE	SIZE	REINFORCEMENT
SF1	24" x 8"	3-15M CONT. BOTTOM



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Discipline: STRUCT. Sr. Reg. No.: 23928 Signature: [Signature]

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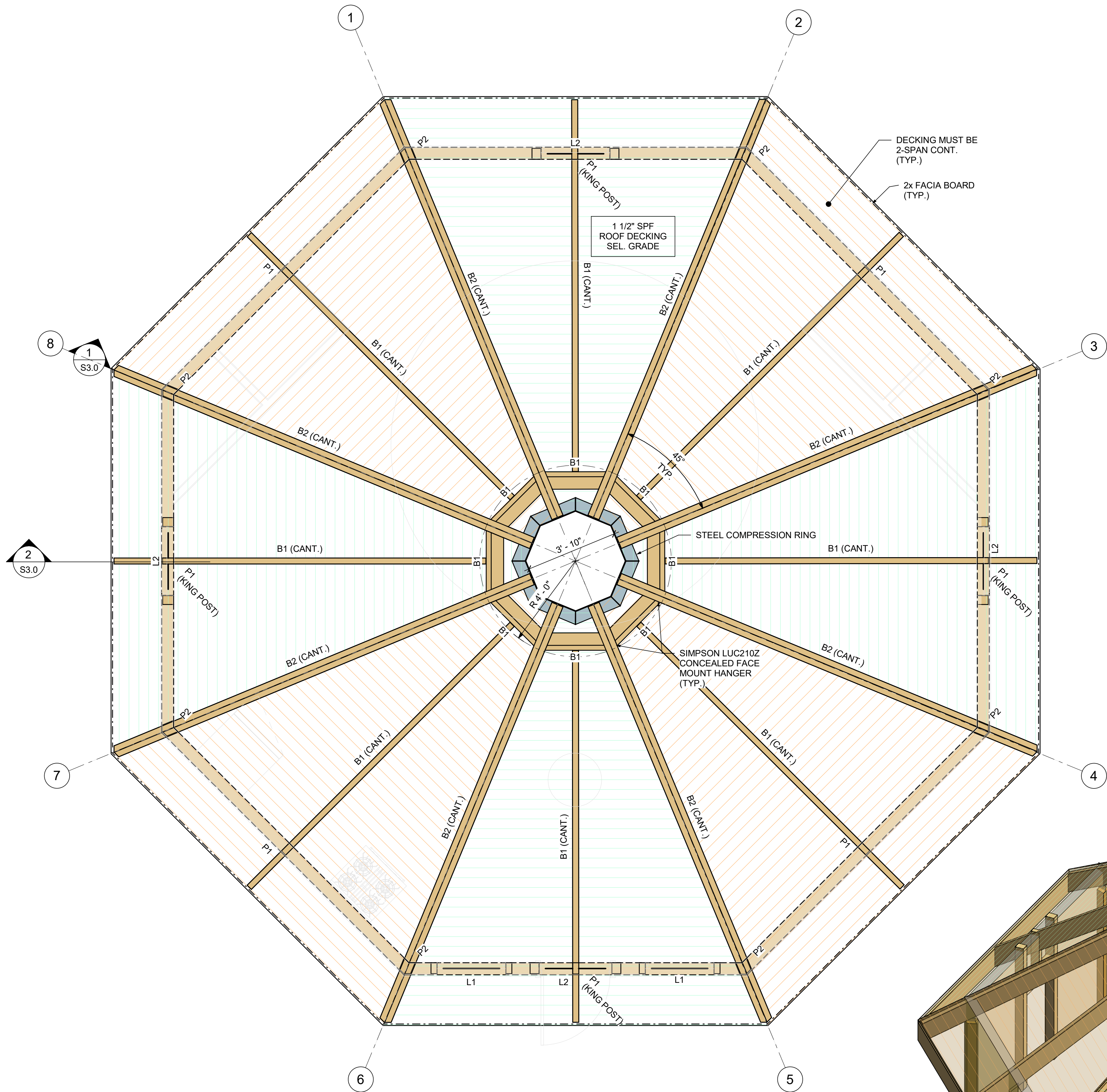
Project
SWEATLODGE
PRINCE ALBERT, SASKATCHEWAN

Title
FOUNDATION PLAN

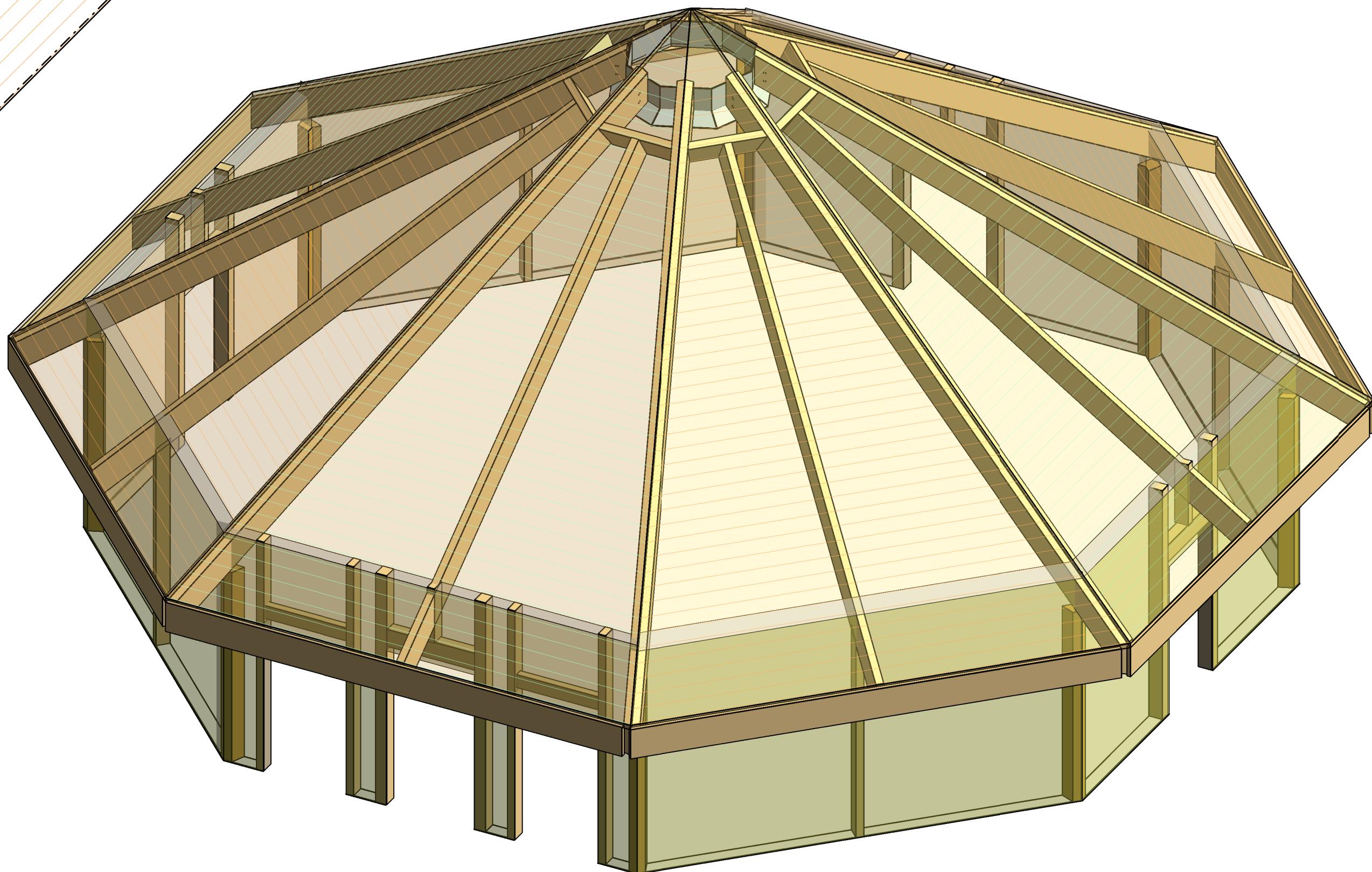
Scale	As indicated	Revision No.	2
Project No.	16918	Drawn:	TD
Design:	DW	Designed:	DW
Design Checked:	TH	Date	Issue Date
Discipline Review:	Date	Issue Date	

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ROOF FRAMING PLAN
3/8" = 1'-0"



PLAN NOTES

1. SEE ARCH. DWG'S FOR DIMENSIONS
2. ALL WALLS TO BE 2x6 STUD WALL @16" O/C U.N.O.
3. PROVIDE 1/2" PLYWOOD WALL SHEATHING
4. ALL BUILT UP COLUMNS AT BEAM BEARING MUST BE CONTINUOUS FROM POINT OF BEARING THERE TO CONCRETE
5. BUILT UP COLUMNS MAY ONLY BE CALLED UP ON PLAN WHERE BEARING OCCURS BUT ARE CONTINUOUS AS PER NOTE 4.

COLUMN SCHEDULE

TYPE	DESCRIPTION
P1	2 - 2x6 SPF No.1/2
P2	4 - 2x6 SPF No.1/2

BEAM SCHEDULE

TYPE	DESCRIPTION
B1	2 PLY 2x12 HEM-FIR No.1/2
B2	4 PLY 2x12 HEM-FIR No.1/2

LINTEL SCHEDULE

TYPE	DESCRIPTION	POST
L1	2-2x6 SPF No.1/2	1 CRIPPLE +1 KING
L2	3-2x8 SPF No.1/2	2 CRIPPLE +1 KING



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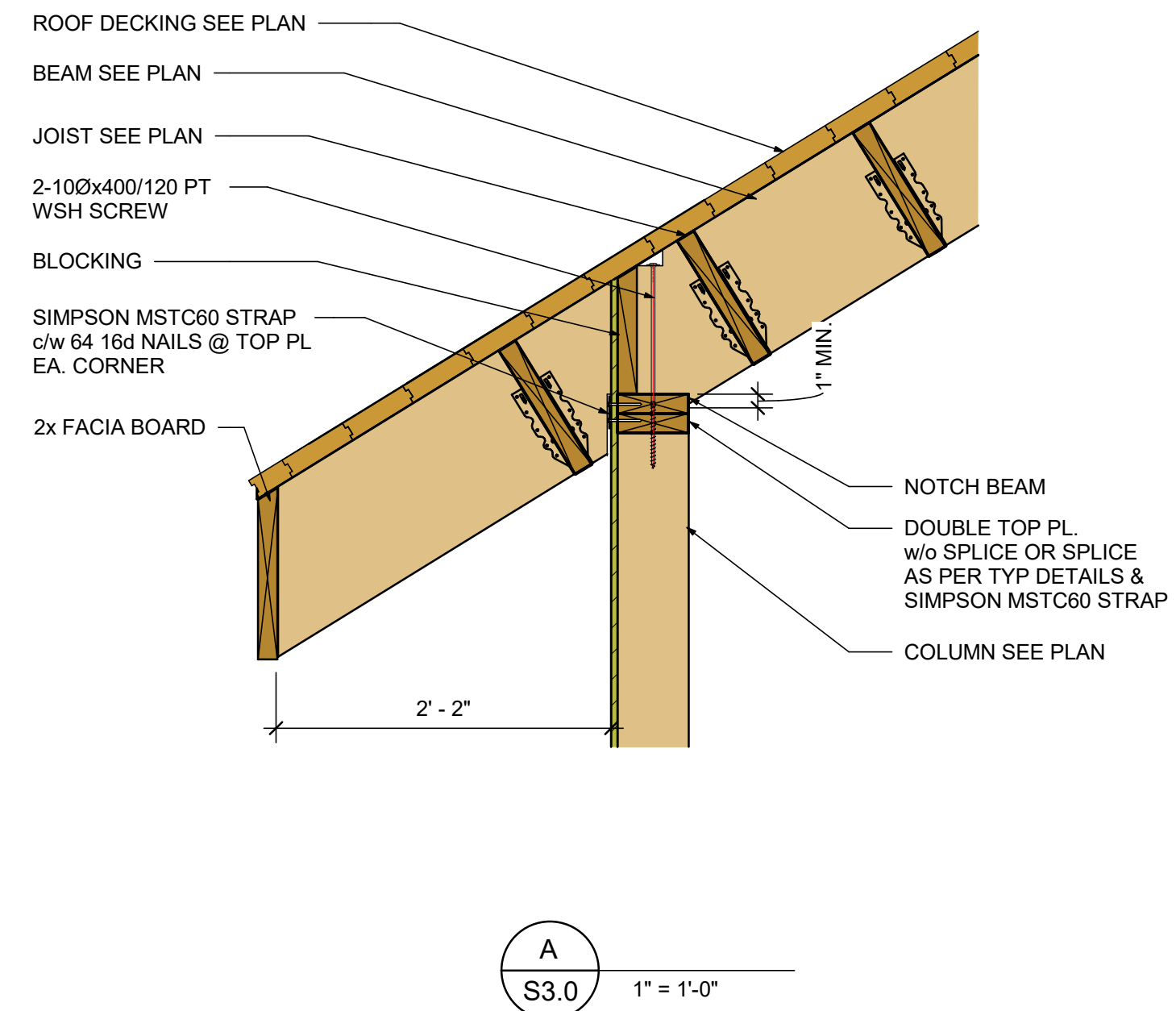
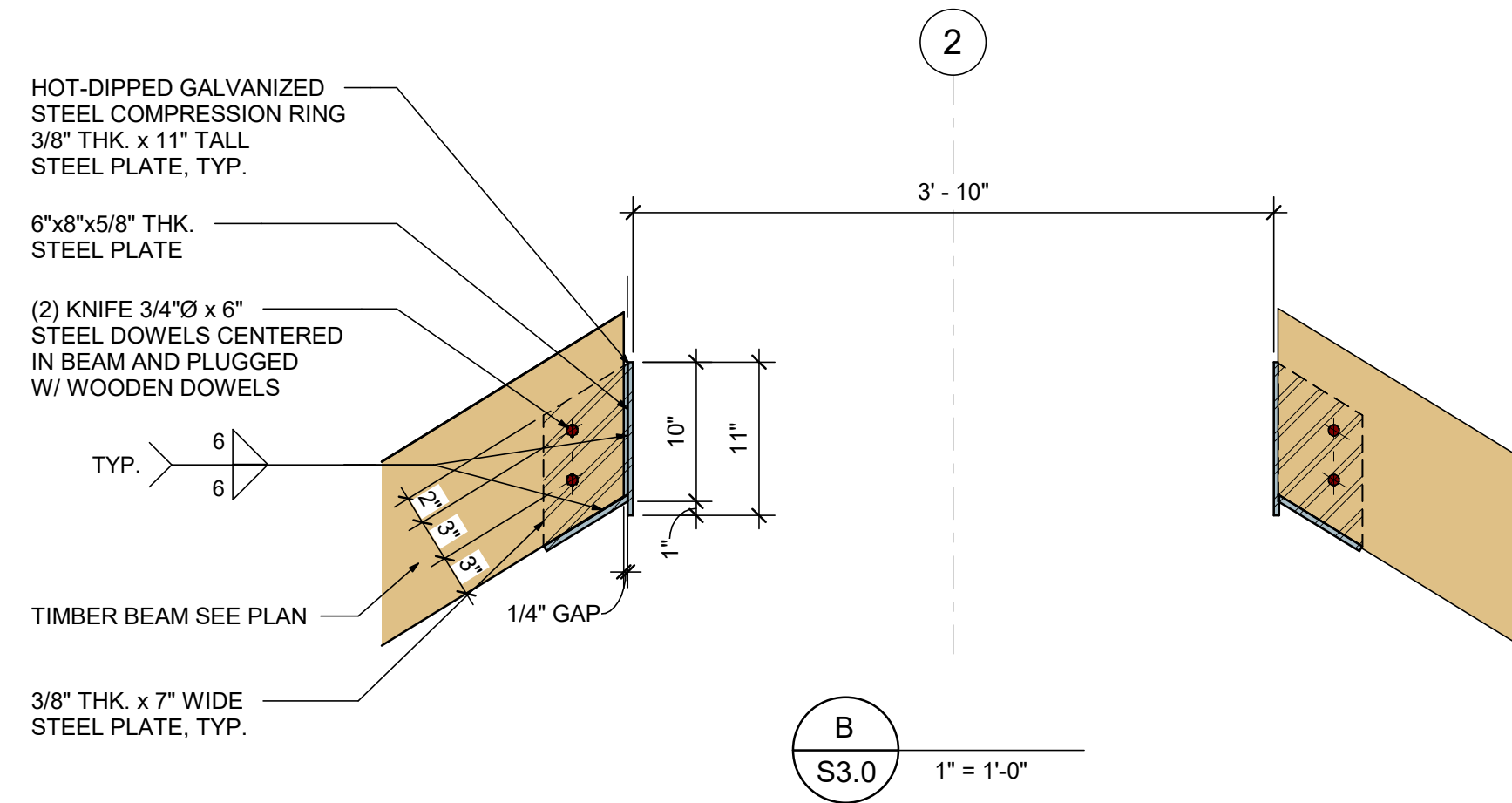
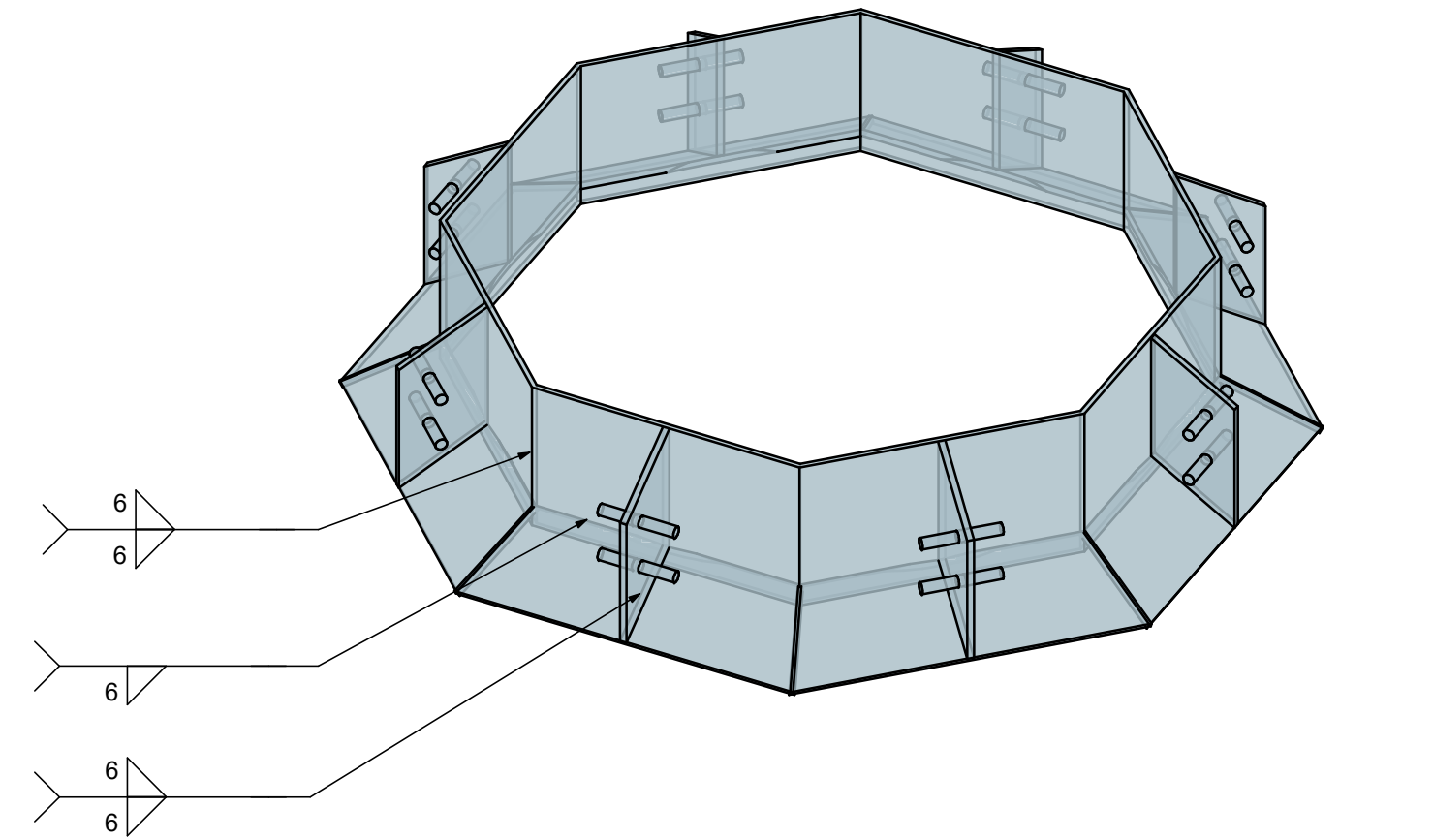
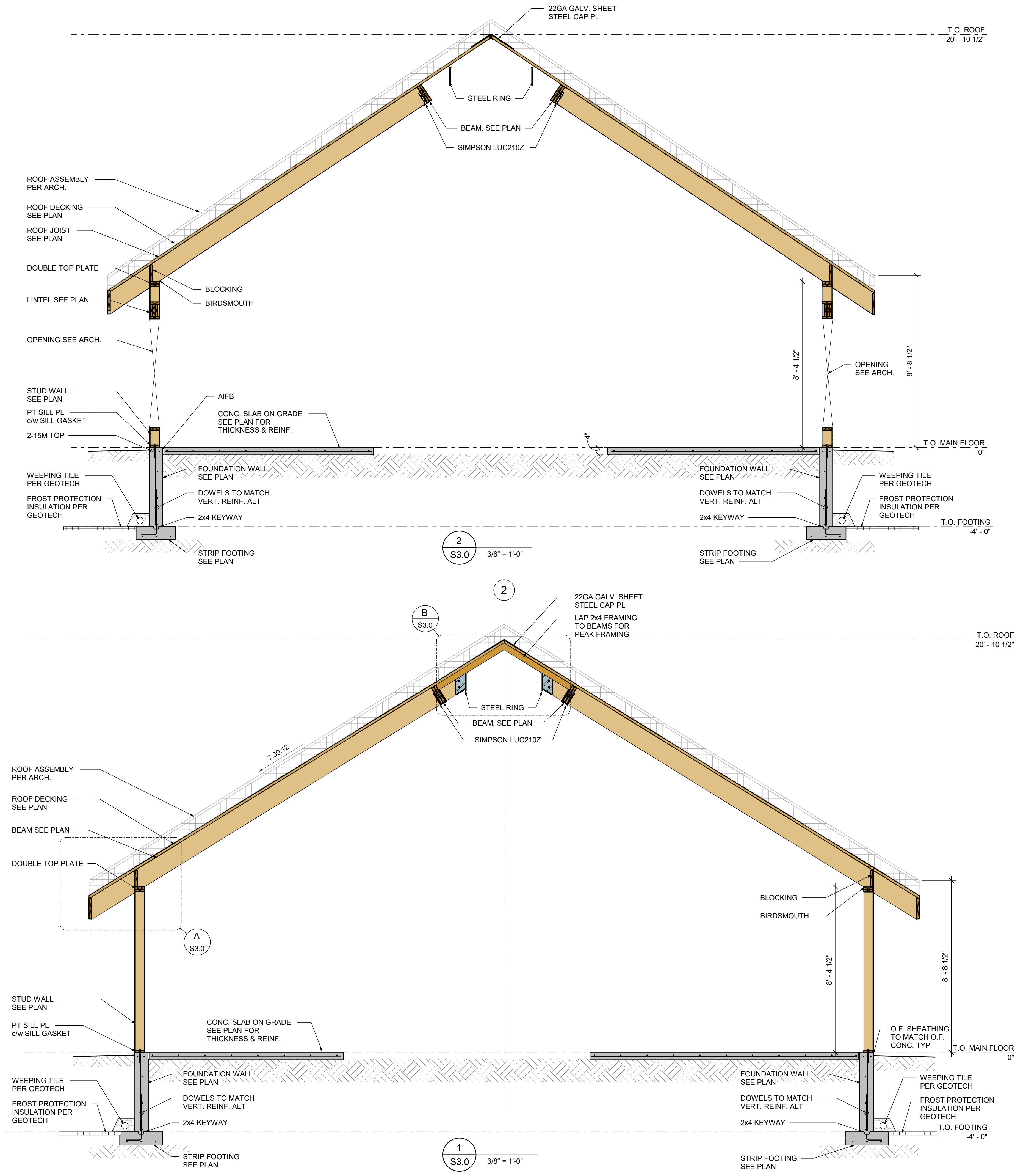
REVISIONS

Project
SWEATLODGE
PRINCE ALBERT, SASKATCHEWAN

Title
ROOF FRAMING PLAN

Scale	As indicated	Revision No.	2
Project No.	16918	Drawing No.	S2.1
Drawn:	Author		
Designed:	Designer		
Design Checked:	Checker	Date	Issue Date
Discipline Review:		Date	Issue Date

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YR. MR. DAY

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Project

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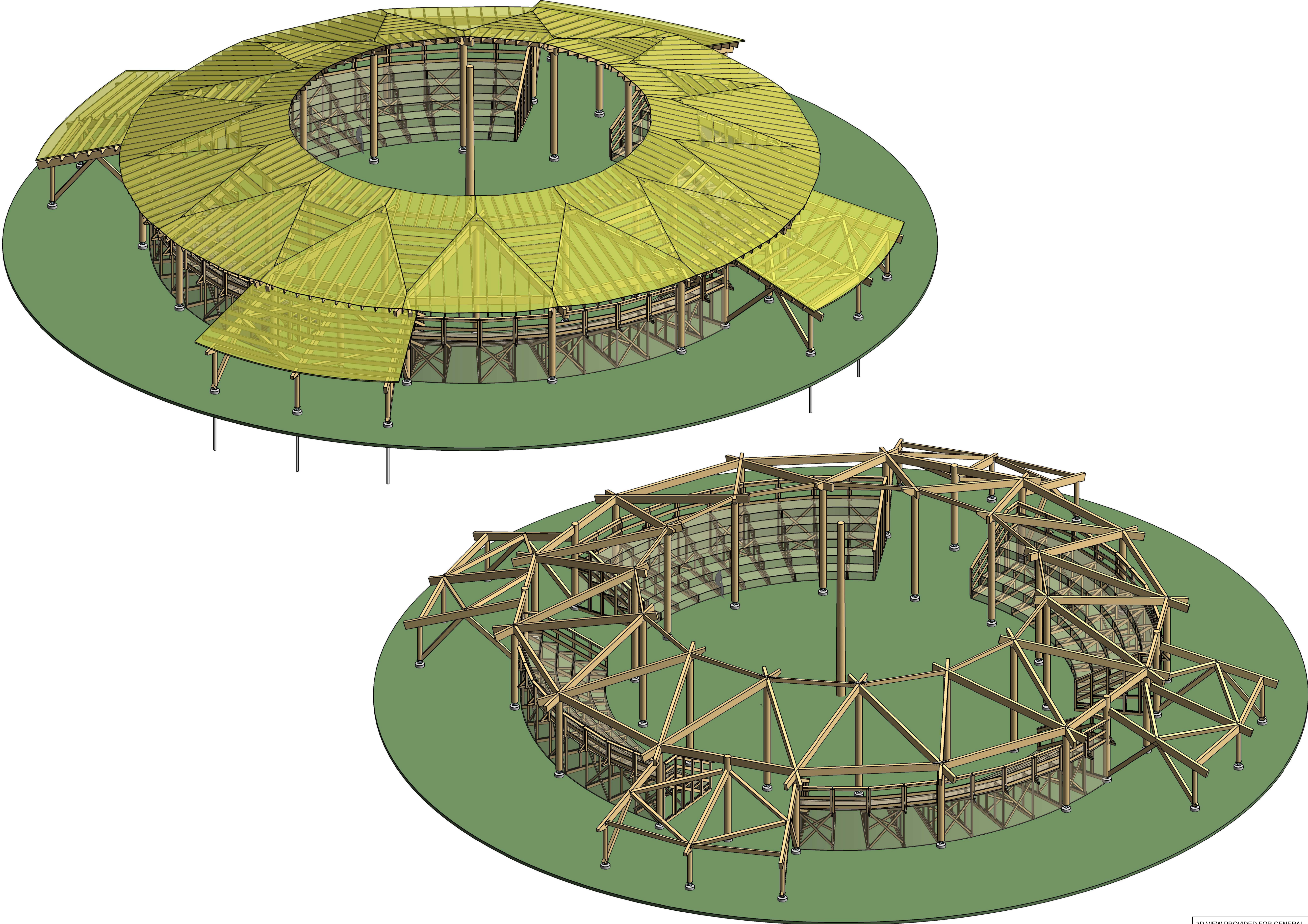
**SECTIONS &
DETAILS**

Scale	As indicated	Revision No.	2
Project No.	16918	Drawn:	TD
Designed:	DW	Design Checked:	TH
Discipline Review:		Date	Issue Date

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PAIMFC ARBOUR

PRINCE ALBERT, SASKATCHEWAN



3D VIEW PROVIDED FOR GENERAL
ORIENTATION OF STRUCTURE ONLY



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SOLO
Situatd On Land Office

Saskatchewan Office 306.280.5588
CHRISTOPHER LAKE, SASKATCHEWAN

P.Eng.

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1	ISSUED FOR REVIEW	DW	2025.04.04

No.	DESCRIPTION	BY	DATE
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Project

PAIMFC ARBOUR

PRINCE ALBERT, SASKATCHEWAN

Title

COVER SHEET

Scale	Revision No.
Project No. 16918	2
Drawn: TD	Drawing No.
Designed: DW	
Design Checked: TH Date 2024.04.02	S0.0
Discipline Review: Date 2024.04.02	

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GENERAL

1. THE CONTRACTOR SHALL EXAMINE ALL DRAWINGS, CHECK DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER FOR CLARIFICATION PRIOR TO COMMENCING CONSTRUCTION. DISCREPANCIES NOT REPORTED ARE THE RESPONSIBILITY OF THE CONTRACTOR. CHECK AND VERIFY ALL DIMENSIONS BEFORE COMMENCING WITH ANY WORK. NOTIFY THE ENGINEER OF ANY ERRORS OR OMISSIONS.
2. TEMPORARY SUPPORT AND TEMPORARY AND PERMANENT BRACING OF LOAD BEARING AND NON-LOAD BEARING ELEMENTS DURING CONSTRUCTION TO RESIST DEAD, LIVE, ENVIRONMENTAL AND CONSTRUCTION LOADS IS THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN OF THE TEMPORARY SUPPORTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
3. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
4. FIELD REVIEWS :
NOTIFY THE ENGINEER 48 HOURS IN ADVANCE FOR FIELD REVIEWS AND APPROVAL OF THE FOLLOWING:
* CONCRETE REINFORCEMENT BEFORE EACH CONCRETE POUR
* WOOD FRAMING BEFORE COVERING
5. DO NOT SCALE FROM THESE DRAWINGS.
6. THE DESIGN HAS BEEN PREPARED BASED ON THE ASSUMPTION THAT THE OWNER AND/OR OPERATOR HAS A SITE SAFETY PLAN IN PLACE TO ADDRESS AND MITIGATE SAFETY HAZARDS, BOTH COMMON AND SPECIFIC TO THIS PROJECT.
7. LOCATING STRUCTURE ON SITE, EXCAVATION AND COORDINATION WITH UTILITIES IS THE RESPONSIBILITY OF THE OWNER.

TIMBER JOINERY SPECIFICATIONS

1. IT IS THE FABRICATOR'S RESPONSIBILITY TO CHOOSE TIMBERS SUITABLE FOR THE APPLICATION AND JOINERY.
2. TIMBERS WILL SHRINK AFTER INSTALLATION UNTIL THEY REACH THE EMC (EQUILIBRIUM MOISTURE CONTENT), THIS WILL CAUSE THE LOOSENING OF BOLTS AND THREADED CONNECTORS. PERIODIC TIGHTENING OF THESE CONNECTIONS BY THE END USER DURING THE FIRST SEVERAL YEARS OF THE STRUCTURE'S SERVICE LIFE MAY BE REQUIRED.
3. NORMAL CONSTRUCTION TOLERANCES/DIMENSIONAL VARIATIONS CAN BE EXPECTED IN THE COMPLETED STRUCTURE AS WELL AS DIMENSIONAL CHANGES DUE TO SHRINKAGE AND SWELLING THROUGHOUT THE LIFE OF THE STRUCTURE. FOR THESE REASONS, DETAILS OF ADJOINING SURFACES AND MATERIALS MUST BE ABLE TO ACCOMMODATE THESE VARIATIONS AND CHANGES. EXPECT JOINERY TO "OPEN" SOMEWHAT DUE TO SHRINKAGE FROM INITIAL MOISTURE CONTENT AT THE TIME OF FABRICATION UNTIL EMC IS REACHED.
4. THE KEY TO THE LONG TERM DURABILITY OF THE STRUCTURE DEPENDS ON KEEPING THE TIMBERS SEALED, DRY AND WELL VENTILATED. THESE DESIGN DETAILS ARE THE RESPONSIBILITY OF OTHERS.

DESIGN DATA

1. DESIGN CODE: 2020 NATIONAL BUILDING CODE (NBC).
2. MATERIAL DESIGN STANDARDS TO:
CSA A23.3-19 DESIGN OF CONCRETE STRUCTURES.
CSA S16-14 LIMIT STATES DESIGN OF STEEL STRUCTURES.
CSA O86-19 ENGINEERING DESIGN IN WOOD.
3. CLIMATIC LOAD DATA AS PER 2020 NBC FOR THE LOCATION OF PRINCE ALBERT
4. THE BUILDING IS CONSIDERED NORMAL IMPORTANCE CATEGORY.
5. SEE PLAN NOTES FOR SPECIFIC LOAD VALUES.
6. SELF WEIGHT (SWT) IS DUE TO THE WEIGHT OF THE STRUCTURE
7. SUPERIMPOSED DEAD LOADS (SDL) ARE NON-STRUCTURAL DEAD LOADS DUE TO NON-STRUCTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, SUSPENDED EQUIPMENT, PAVERS, SOIL, ETC.
8. DEAD LOAD (DL) IS THE SELF WEIGHT OF THE STRUCTURE PLUS THE SUPERIMPOSED DEAD LOAD.
9. UNLESS OTHERWISE NOTED, DESIGN LOADS SHOWN ON DRAWINGS ARE SPECIFIED (UNFACTORED) LOADS, TO BE USED FOR ULS DESIGN. FOR SLS DESIGN, THESE LOADS CAN BE REDUCED BY MULTIPLYING WITH THE RATIO OF APPROPRIATE IMPORTANCE FACTORS Ix(SLS) / Ix(ULS) GIVEN BELOW.
10. LIVE LOAD 4.8 kPa (BLEACHERS)
11. SNOW: Ss = 1.90 kPa , Sr = 0.1 kPa , Is (ULS) = 1.0 , Is (SLS) = 0.9
12. RAIN: 24 HOUR RAINFALL = 81 mm
13. WIND: q50 = 0.38 kPa, Iw(ULS) = 1.0, Iw(SLS) = 0.75
BUILDING IS: LOW RISE
TERRAIN TYPE: OPEN
WIND LOADING FOR OPEN STRUCTURES TO ASCE 7
14. LATERAL LOADS RESISTING SYSTEM: TIMBER BRACE FRAMES
15. BUILDING MEETS Cl.4.1.8.1 (2) OF 2020 NATIONAL BUILDING CODE
SEISMIC: Sa(0.2) = 0.0835
Sa(0.5) = 0.0502
Sa(1.0) = 0.0259
Sa(2.0) = 0.0112
PGA = 0.0366
SITE CLASSIFICATION = C
Ie = 1.0
Fs = 1.0, Rs = 1.5, Ts = 0.34
16. DEFLECTION LIMITS:
ROOF & FLOORS, TOTAL LOAD = SPAN / 180
FLOORS & ROOFS, LIVE LOAD = SPAN / 360
ROOF, SNOW LOAD = SPAN / 240
LATERAL STOREY DRIFT = HEIGHT / 500 (WIND)
= HEIGHT x 0.02 (SEISMIC)

STEEL SCREW PILE NOTES

1. THE DESIGN, FABRICATION, AND INSTALLATION OF PILES SHALL BE GOVERNED BY THE FOLLOWING CODES:
 - CSA G40.21 STRUCTURAL QUALITY STEELS
 - CSA W48.14 MILD STEEL COVERED ARC-WELDING ELECTRODES
 - CSA W48.5 MILD STEEL ELECTRONICS FOR FLUC-CORE ARC-WELDING
 - CSA W47.1-09 CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL STRUCTURES
 - PROVINCIAL OCCUPATIONAL HEALTH AND SAFETY REGULATIONS
 - ASTM A252 WELDED AND SEAMLESS STEEL PIPE PILES
 - ASTM A53 WELDED AND SEAMLESS PIPE
2. PILES SHALL BE DESIGNED FOR FACTORED LOAD SHOWN ON PLAN.
3. PILE WALL THICKNESS AND SHALL BE INCREASED AS NECESSARY TO ACCOMMODATE INSTALLATION EQUIPMENT.
4. SCREW PILES SHALL BE PLACED NOT CLOSER THAN 3 HELIX DIAMETERS FROM ADJACENT PILE U.N.O. PILES SHALL BE PLACED WITHIN 38mm (1 1/2") OF DESIGN DRAWINGS AND SHALL BE PLUMB TO 38mm (1 1/2"). SCREW PILES NOT WITHIN TOLERANCE MAY REQUIRE LOAD TESTING AT STRUCTURAL ENGINEERS DISCRETION.
5. SPACING OF HELICES SHALL NOT BE LESS THAN 3 TIMES THE HELIX DIAMETER.
6. SPACING OF HELICES SHALL BE A WHOLE NUMBER MULTIPLE OF THE HELIX PITCH.
7. SUBMIT SCREW PILE LAYOUT AND SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF JURISDICTION.
8. SCREW PILES & PILE CAPS TO BE BARE STEEL.

MATERIAL SPECIFICATIONS

- WOOD: TO CONFORM WITH CSA/CAN O86.19 ENGINEERING DESIGN IN WOOD
- FRAMING

1. GRADES AND TYPES TO BE AS FOLLOWS U.N.O.:
 - WOOD BASED SHEATHING: ORIENTED STRANDBOARD TO CAN-0473.0-93 GRADE 0-2. PLYWOOD TO CSA 0325-07
 - STRUCTURAL JOISTS & PLANKS: SPF #2 OR BETTER
 - STRUCTURAL LIGHT FRAMING: SPF #2 OR BETTER
 - WOOD POSTS & BUILT UP COLUMNS: SPF #2 OR BETTER
- TIMBER

1. TO MEET NLGA VISUAL GRADING RULES U.N.O.:
SPECIES: COLUMNS & VERTICAL BRACING: S-P-F LODGEPOLE PINE "PINUS CONTORTA" S-P-F BEAMS & HOR. BRACING: S-P-F No.1 OR BETTER

2. FINISH SIZE: NOMINAL DIMENSIONS (I.e. 8x10 = 7-1/2"x9-1/2") DRY SERVICE CONDITION (MC<19%)

3. APPLY RUBIO MONOCOAT EXTERIO HARDWAX OIL FINISH TO ALL LOGS/TIMBERS/GLT OR APPROVED EQUIVALENT (TINT/COLOUR TO BE APPROVED BY CLIENT).
- HARDWARE

1. TO CONFORM TO THE FOLLOWING U.N.O.:
 - BOLTS WOOD TO WOOD & STEEL: ASTM A307
 - BOLTS STEEL TO STEEL: ASTM A325
 - THREADED ROD: F1554 Grade 36
 - DRIFT PINS: ASTM A307 OR CSA/CAN-G40.21 300W.
- CUSTOM HARDWARE

1. FABRICATE TO CAN/CSA S16.14.

2. PROVIDE STEEL TO CSA/CAN-G40.20/G40.21 OR ASTM STANDARD A992/A992M. THE FOLLOWING SHOULD HAVE GRADES MINIMUM OF:
 - STRUCTURAL SHAPES 350W
 - HOLLOW STRUCTURAL SECTIONS 350W
 - COLUMN BASE PLATES 300W
 - MISCELLANEOUS PLATES 300W

3. PIPE SECTIONS ASTM A53, 241W

4. HOT DIP GALVANIZE ALL HARDWARE. ALL FINISHES TO CLIENT OR ARCHITECTS SPECIFICATIONS.
- CONCRETE REINFORCEMENT

1. PROVIDE REINFORCEMENT TO CAN/CSA-A23.3 AND CSA G30.18 AS FOLLOWS: 400 MPa FOR 15M OR LARGER

2. PROVIDE NEW DEFORMED BARS TO CSA G30.18 GRADE 400.

CAST-IN-PLACE CONCRETE

1. CONCRETE AS PER CSA A23.1 ALTERNATIVE 1 - PERFORMANCE SPECIFICATION.
2. CONTRACTOR AND CONCRETE SUPPLIER TO ENSURE THAT MIX PROPERTIES MEET REQUIREMENTS FOR PLACING, FINISHING AND THE SPECIFIED PERFORMANCE.
3. CONCRETE SUPPLIER TO BE CERTIFIED BY THE ALBERTA READY MIXED CONCRETE ASSOCIATION OR SASKATCHEWAN READY MIXED CONCRETE ASSOCIATION.
4. PORTLAND CEMENT TYPE GU UNO OR REQUIRED BY EXPOSURE CLASS.
5. CONCRETE TO BE NORMAL DENSITY (MIN. 2300 kg/m³) UNLESS NOTED OTHERWISE.
6. NOMINAL MAXIMUM SIZE OF COARSE AGGREGATE TO BE 20 (3/4") UNLESS NOTED OTHERWISE.
7. CONCRETE MIX SCHEDULE:

ELEMENT	COMPRESSIVE STRENGTH (MPa) AT 28 DAYS	EXPOSURE CLASS	REMARKS
PILES CAPS	35	S2	

- NOTES:
1. F-2 FOR ELEMENTS SUSCEPTIBLE TO FREEZING. N EXPOSURE CLASS FOR ELEMENTS NOT SUSCEPTIBLE TO FREEZING.
2. SPECIFIED CONCRETE STRENGTH AT 56 DAYS FOR CLASS S EXPOSURE CONCRETE.
8. REFER TO CSA A23.1 FOR THE MAXIMUM WATER/CEMENT RATIO, MINIMUM COMPRESSIVE STRENGTH, AIR CONTENT, CURING REQUIREMENTS, CHLORIDE ION PENETRABILITY AND ALTERNATE CEMENT TYPES TO MEET THE REQUIREMENTS FOR THE NOTED EXPOSURE CLASS.
9. DO NOT ADD WATER TO CONCRETE ON SITE.
10. VIBRATE ALL CONCRETE.
11. PROTECT CONCRETE FROM FREEZING. DO NOT PLACE CONCRETE AGAINST FROZEN GROUND. USE COLD WEATHER CONCRETING METHODS IN ACCORDANCE WITH CSA-A23.1 WHEN TEMP IS BELOW +5°C. MAINTAIN CONCRETE TEMPERATURE OF 10° IS MAINTAINED THROUGHOUT THE CURING PERIOD (MINIMUM 3 DAYS).
12. PROTECT CONCRETE FROM EXCESSIVE HEAT AND DRYING. USE HOT WEATHER CONCRETING METHODS IN ACCORDANCE WITH CSA-A23.1 WHENEVER THE OUTDOOR TEMPERATURE IS GREATER THAN 27°C.
13. TESTING

1. PROVIDE INSPECTION REPORTS PREPARED BY AN INDEPENDENT INSPECTION AND TESTING AGENCY AT THE COST OF THE CONTRACTOR.

2. ONE STANDARD TEST FOR EACH 75 CUBIC METERS OF CONCRETE, BUT NOT LESS THAN ONE TEST FOR CONCRETE CAST EACH DAY. THREE CONCRETE CYLINDERS FOR EACH TEST IN ACCORDANCE WITH CSA A23.1 AND 2.
14. CONCRETE FORMWORK

1. FORMWORK TO CSA S269.1-16. FORMWORK DESIGN IS THE RESPONSIBILITY OF THE CONTRACTOR.

2. COLUMN AND WALL FORMS SHALL REMAIN UNTIL CONCRETE HAS REACHED 15MPa OR MIN 1 DAY.

TIMBER FRAMING

1. CONFIRM ALL DIMENSIONS, CONNECTION GEOMETRY AND CONSTRUCTABILITY OF THE ASSEMBLIES BEFORE PROCEEDING WITH FABRICATION. REPORT DISCREPANCIES TO THE ENGINEER.
2. TIMBER FRAME SUPPLIER TO SUBMIT DIGITAL PDF COPY OF SHOP DRAWINGS FOR REVIEW AND APPROVAL OF THE ENGINEER PRIOR TO PROCEEDING WITH THE FABRICATION.
3. ALL CONNECTIONS TO MEET THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND CSA O86-19 ENGINEERING DESIGN IN WOOD.
4. TEMPORARY SUPPORT AND TEMPORARY BRACING OF THE TIMBER ELEMENTS DURING ERECTION AND CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. UNLESS NOTED THE TENONS ARE TO BE MINIMUM OF 2 1/2" THK.
6. HOUSINGS ARE TYPICALLY 1/16" DRAUGHT BORED.

SELF-TAPPING TIMBER FASTENERS

1. TIMBER FASTENERS ARE IDENTIFIED ON STRUCTURAL DRAWINGS IN THE FOLLOWING FORMAT:

- [DIAMETER] X [LENGTH] / [THREAD LENGTH] [THREAD TYPE] [HEAD TYPE]

FT = FULLY THREADED
PT = PARTIALLY THREADED

HEAD TYPES:
CSK = COUNTERSINK HEAD
CYL = CYLINDER HEAD
HEX = HEX HEAD
WSH = WASHER/PAN HEAD

EXAMPLE: 2 - 100 X 200/100 PT CSK
= TWO, 10MM DIAMETER X 200MM LONG SCREWS WITH 100MM THREAD, PARTIALLY THREADED WITH COUNTERSINK HEAD

2. THE FOLLOWING TIMBER SCREWS ARE ACCEPTABLE UNLESS A SPECIFIC MANUFACTURER IS NOTED ON DRAWINGS. APPROVED PRODUCTS NOTED BELOW ARE ACCEPTABLE BASED ON THEIR MOST CURRENT CCMC OR ETA.
- 2.1. PARTIALLY THREADED SELF TAPPING SCREWS
MANUFACTURER FASTENER TYPE
HECO TOPIX
SFS INTEC SFS WFC-T
SFS WFR-T
SFS WFD-T
SWG (WÜRTH) ASSY 3.0 SK
ASSY 3.0 ECOFAST
ASSY 3.0 KOMBI
T-STAR

SPAX SFS WB
ASSY PLUS VG
T-STAR
- 2.2. FULLY THREADED SELF TAPPING SCREWS
MANUFACTURER FASTENER TYPE
HECO TOPIX CC
SFS INTEC SFS WT-T
SFS WR-T
SFS WB
SWG (WÜRTH) ASSY PLUS VG
SPAX T-STAR
3. SCREW TYPES SPECIFIED ON STRUCTURAL DRAWINGS SUPERSEDE THE INFORMATION ABOVE U.N.O.
4. HOLES IN STEEL PLATE TO MATCH THE SCREW TYPE USED.
5. FOR PARTIALLY THREADED SCREWS, EMBED FULL THREAD INTO SECONDARY MEMBER OR TO A DEPTH AS SPECIFIED ON DRAWINGS.
6. WOOD TO WOOD CONNECTIONS TO HAVE WASHER OR PAN-HEAD U.N.O.
7. WHERE PRE-DRILLING OF SCREWS IS RECOMMENDED BY THE SCREW MANUFACTURER, HOLE DIAMETER TO BE STRICTLY AS PER MANUFACTURER'S RECOMMENDATIONS.
8. DO NOT USE IMPACT DRILLS FOR SCREW INSTALLATION.
9. DO NOT USE A HAMMER TO ENGAGE THE FASTENER WITH THE WOOD.
10. FULLY THREADED FASTENERS TO BE INSTALLED IN A ONE STEP PROCESS. DO NOT STOP THE DRILL DURING THE INSTALLATION.
11. METRIC CONVERSIONS:
DIAMETER 8MM = 5/16"
10MM = 3/8"
12MM = 1/2"
25MM = 1"
SCREW / THREAD LENGTH
ROUND UP TO NEAREST STANDARD LENGTH FOR MANUFACTURER

ROUGH CARPENTRY

1. WOOD FRAMING TO CONFORM TO NLGA STANDARD GRADING RULES FOR CANADIAN LUMBER AND CSA O86-19 ENGINEERING DESIGN IN WOOD
2. THE ROOF SHEATHING AND SUPPORTING ROOF MEMBERS HAVE BEEN DESIGNED AS A DIAPHRAGM. DIAPHRAGM CONNECTION REQUIREMENTS FOR THE ROOF DECKING ARE: 2 1/2" x 3.33mm GUN NAILS @ 6" O/C AT SUPPORTED PANEL EDGE AND AT 12" O/C ALONG INTERMEDIATE FRAMING MEMBERS.
3. WIRE NAILS, SPIKES AND STAPLES TO CSA 19111-1974.

FOUNDATIONS

1. A GEOTECHNICAL INVESTIGATION HAS NOT BEEN COMPLETED FOR THE SITE. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR FOUNDATION CAPACITY.PILES SHALL BE DESIGNED FOR FACTORED LOAD SHOWN ON PLAN.
2. REFER TO THE GEOTECHNICAL REPORT FOR DETAILED INFORMATION ON GEOTECHNICAL CONDITIONS, FOUNDATION RECOMMENDATIONS, AND FOR ALL EARTHWORK INCLUDING EXCAVATION, BACKFILL AND SUBGRADE PREPARATION.
3. LOCARE ALL UNDERGROUND UTILITIES PRIOR TO STARTING ALL FOUNDATION WORK.
4. PILE TOLERANCES:
LOCATION: 75 (3")
CUT OFF ELEVATION: +50 (+2"), -100 (-4")



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REVISIONS			

Project

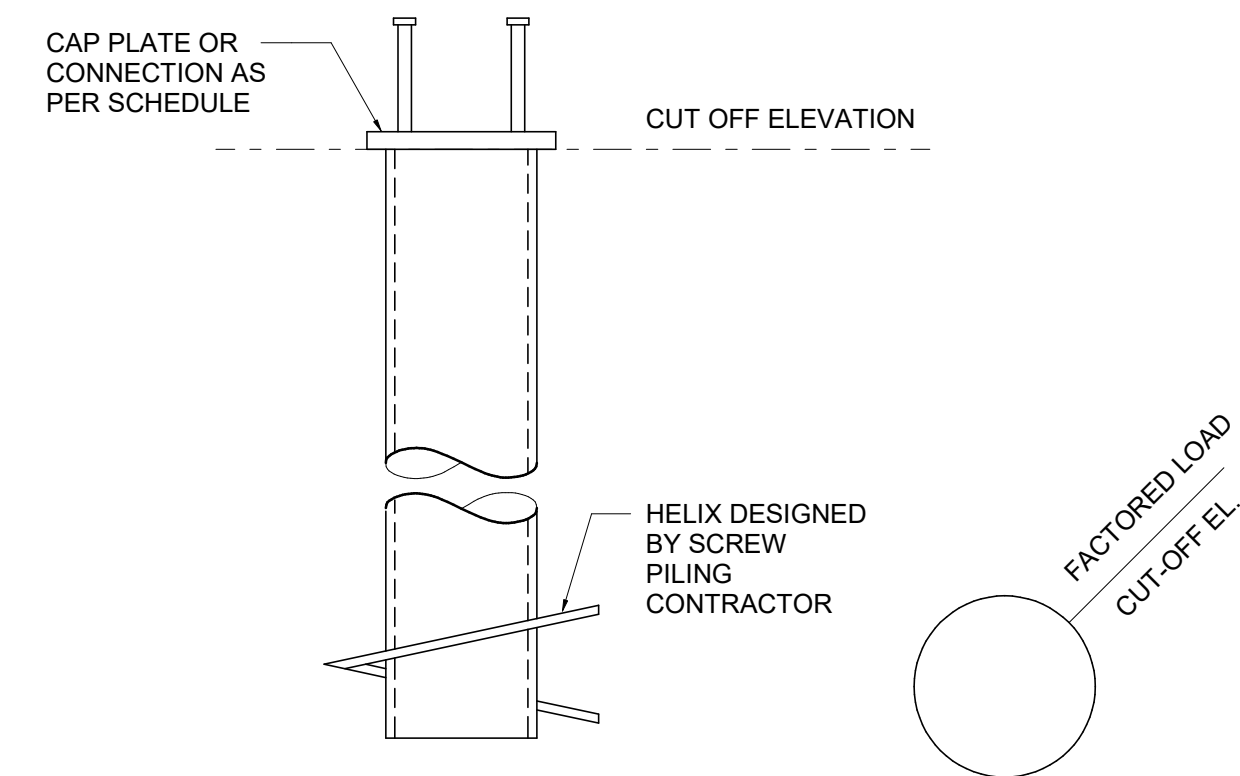
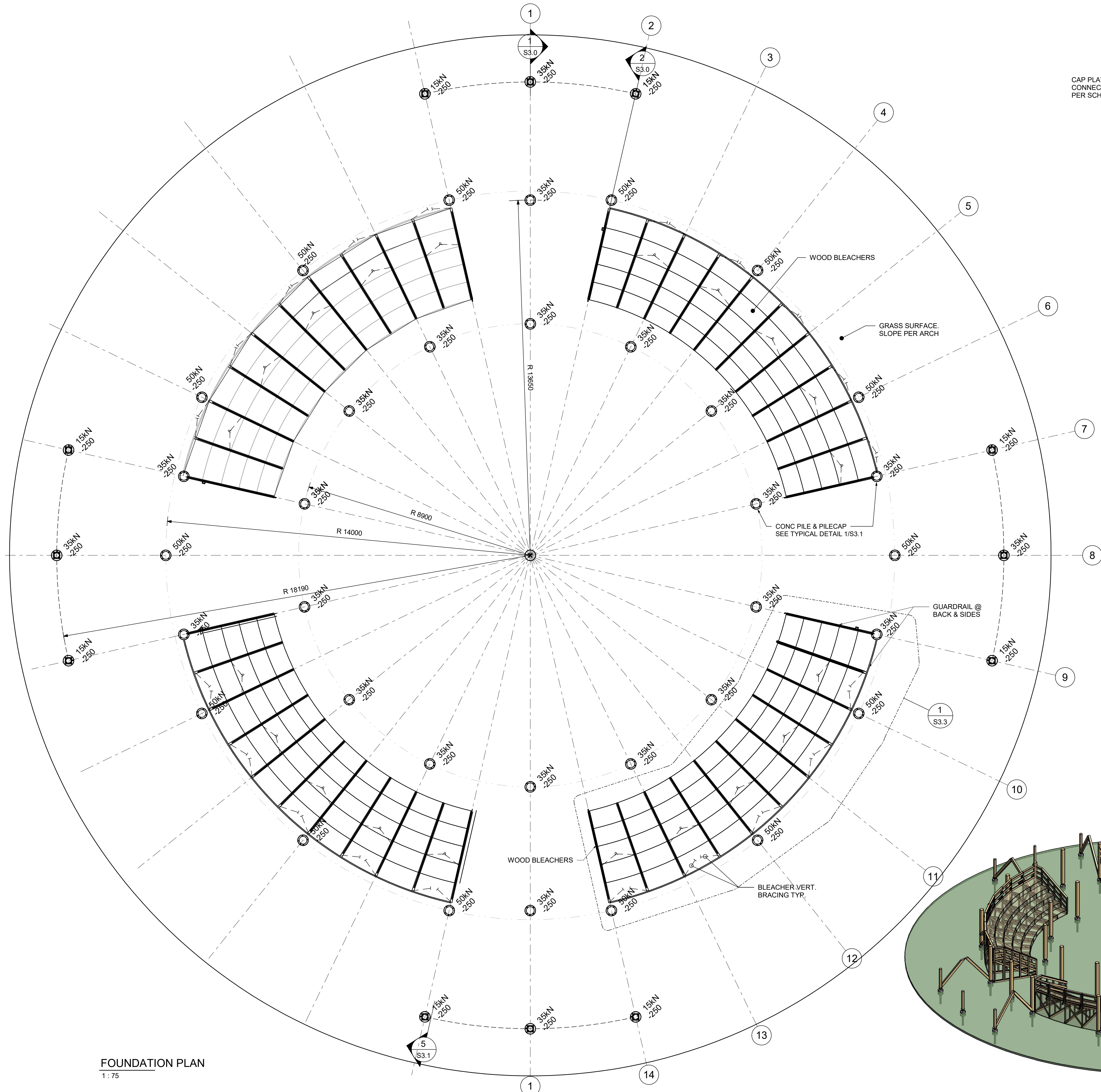
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Title

GENERAL NOTES	
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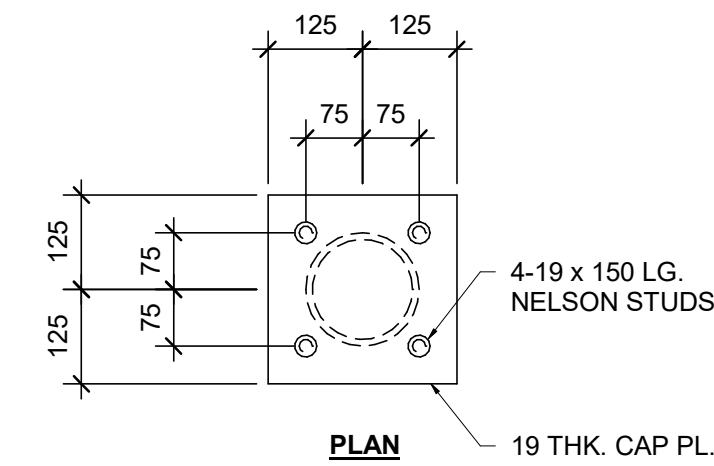
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Project No. 16918	2
Drawn: TD	Drawing No.
Designed: RO	S1.0
Design Checked: TH Date 2024.04.02	
Discipline Review: Date 2024.04.02	

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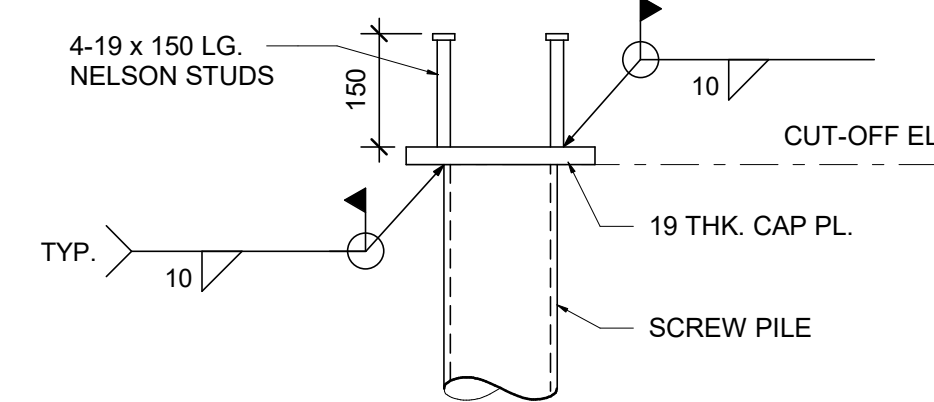


4 TYPICAL SCREW PILE
1 : 10
S2.0

NOTE:
1. DESIGN TYPICAL PILE FOR 5kN FACTORED LATERAL FORCE.
2. DESIGN PILES w/ VERTICAL BRACING FOR 16kN FACTORED LATERAL FORCE.

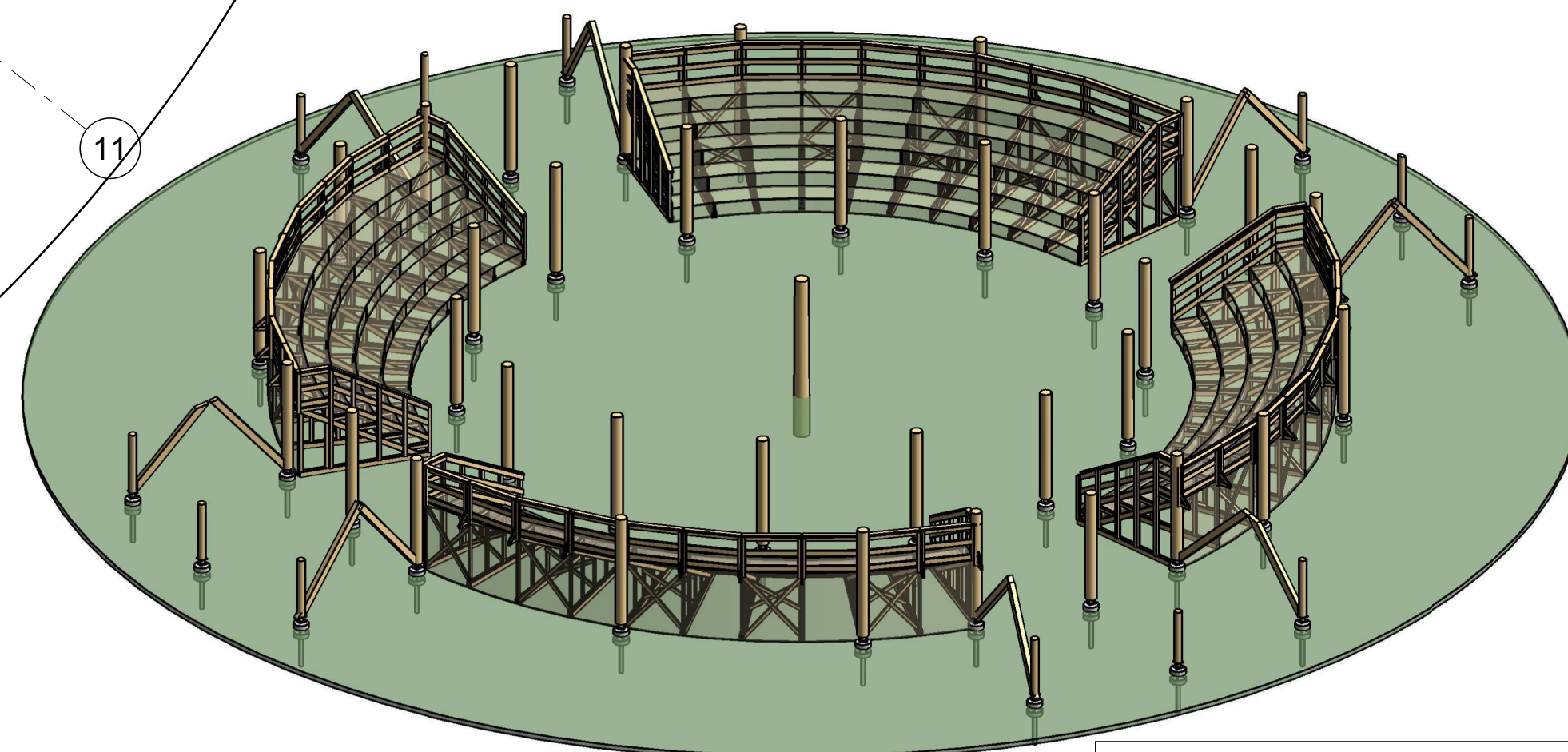


PLAN



SECTION

3 STD-FDN-01B - SCREW PILE CONNECTION
1 : 10
S2.0



3D VIEW ROOF FRAMING PROVIDED FOR
GENERAL ORIENTATION OF STRUCTURE ONLY

FOUNDATION PLAN
1 : 75



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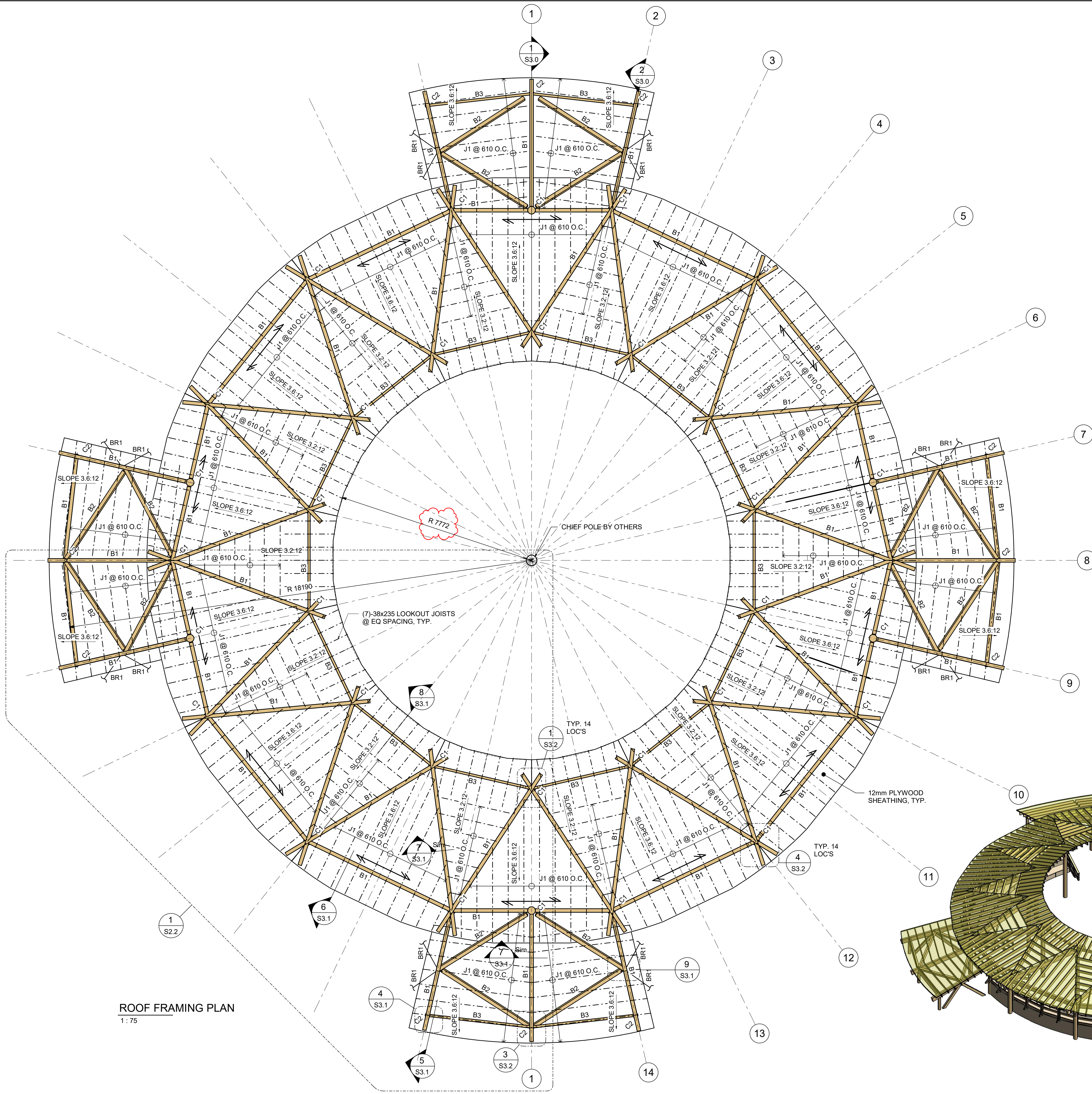
Project
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Title
FOUNDATION PLAN

Scale	As indicated	Revision No.	2
Project No.	16918	Drawn No.	
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ROOF FRAMING PLAN
1:75

JOIST SCHEDULE	
TYPE	DESCRIPTION
J1	38x235 SPF No.1/2

TIMBER POST SCHEDULE	
TYPE	DESCRIPTION
C1	3000 LODGEPOLE PINE "PINUS CONTORTA" SPF No.1 (BARK REMOVED & PEELED W/ DRAW KNIFE - MIN. DIAMETER NOTED)
C2	2000 LODGEPOLE PINE "PINUS CONTORTA" SPF No.1 (BARK REMOVED & PEELED W/ DRAW KNIFE - MIN. DIAMETER NOTED)

TIMBER BEAM SCHEDULE	
TYPE	DESCRIPTION
B1	140x343 SPF No.1
B2	140x191 SPF No.1
B3	89x184 SPF No.1

TIMBER BRACE SCHEDULE	
TYPE	DESCRIPTION
BR1	140x191 SPF No.1



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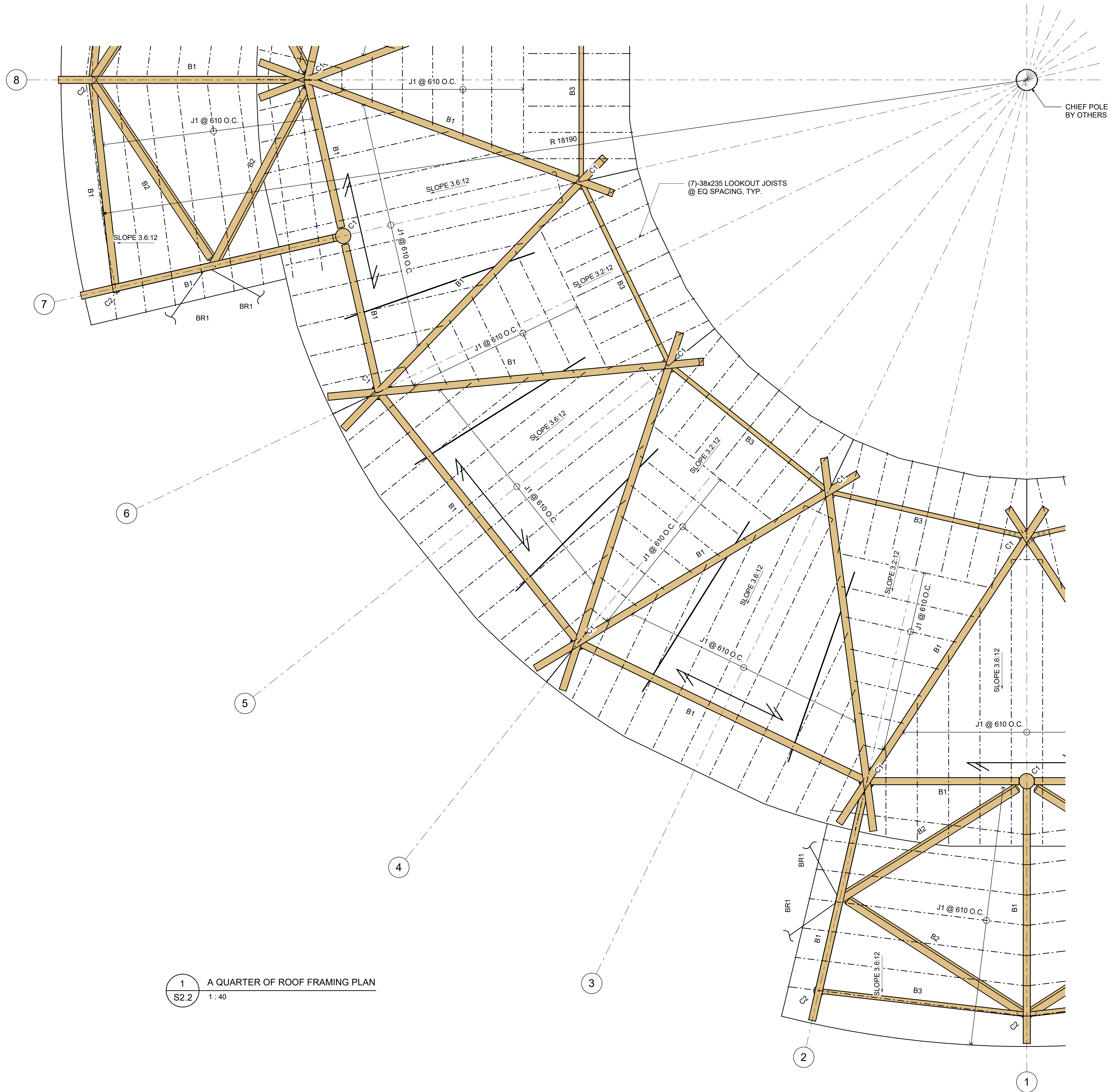
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ROOF FRAMING PLAN

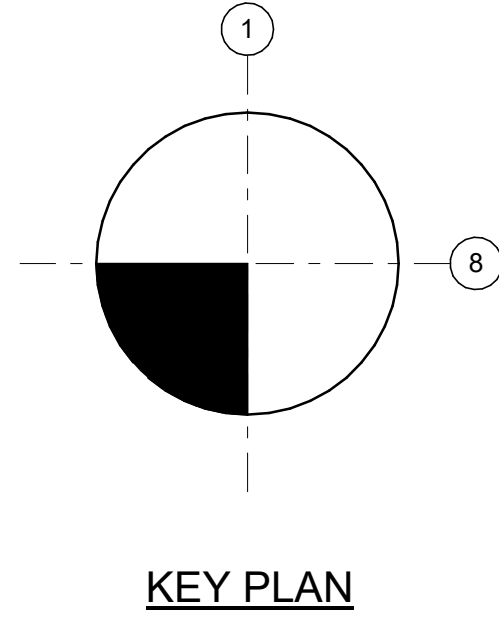
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Project No.	16918	Drawn:	TD
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S2.1

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1
S2.2
A QUARTER OF ROOF FRAMING PLAN
1:40



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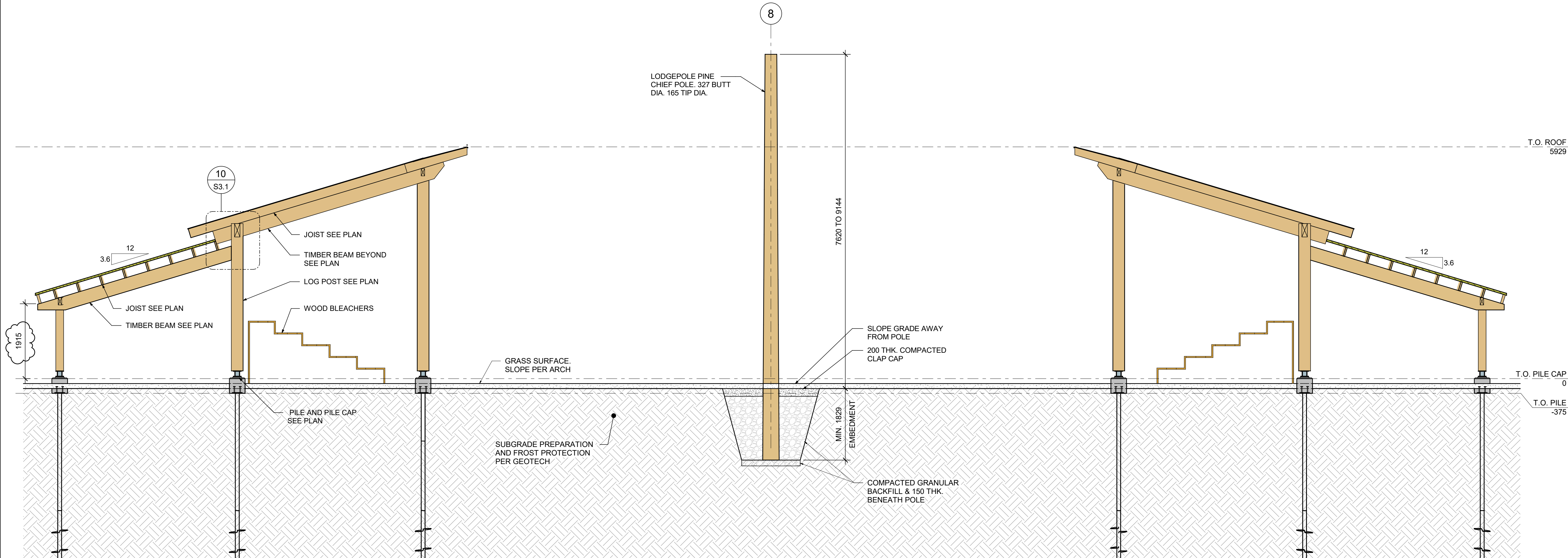
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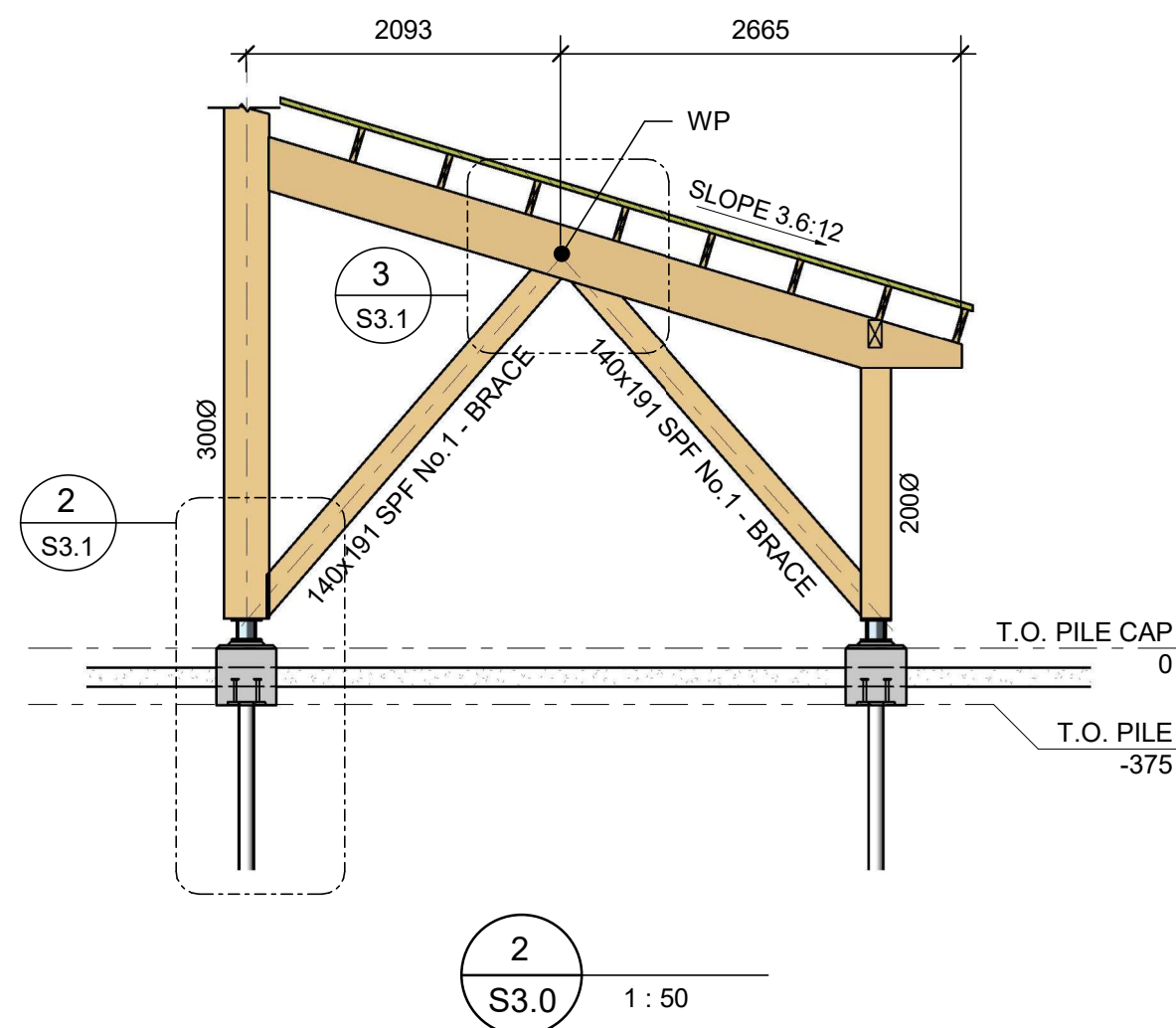
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A QUARTER OF ROOF FRAMING PLAN

Scale	As indicated	Revision No.	2
Project No.	16918	Drawing No.	S2.2
Drawn:	TD		
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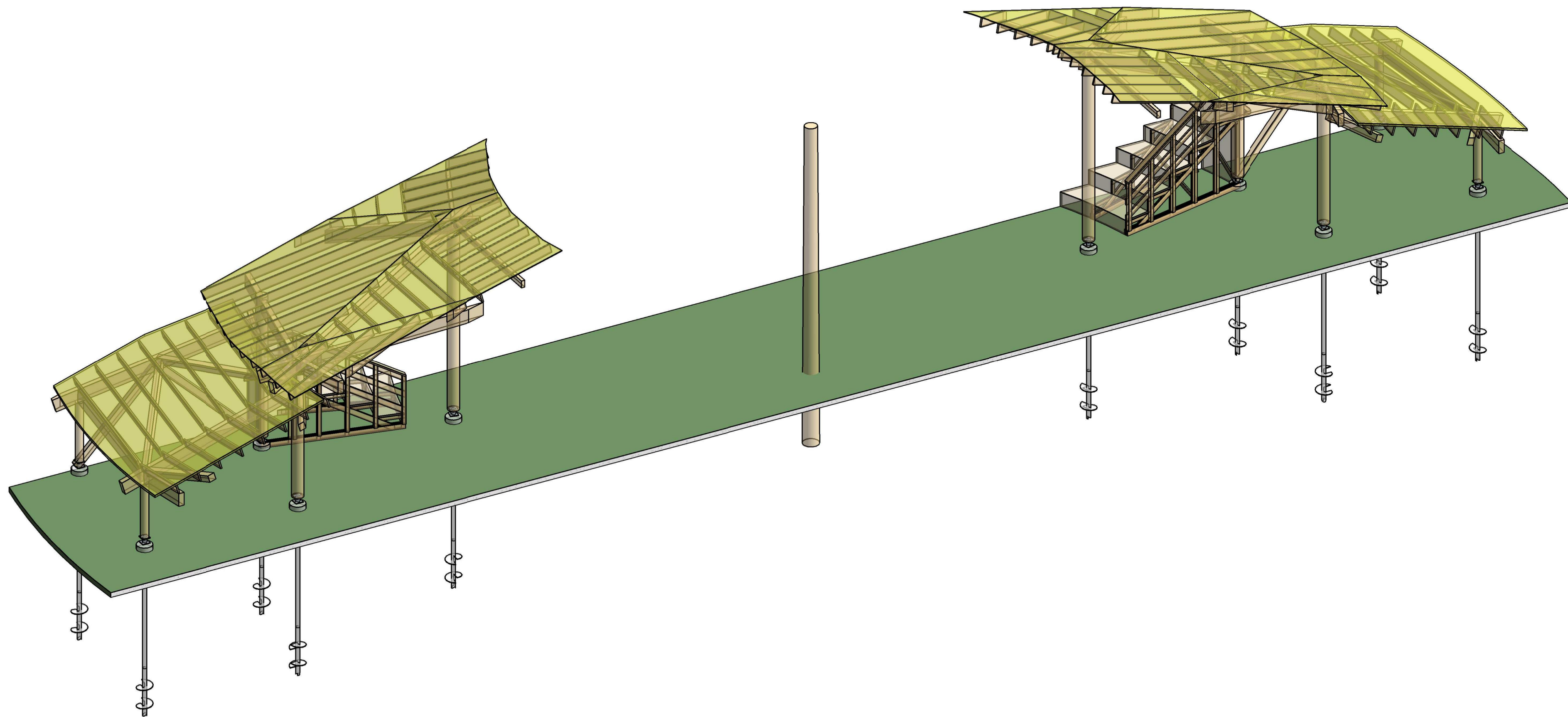
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1
S3.0
BUILDING SECTION
1:55



2
S3.0
1:50



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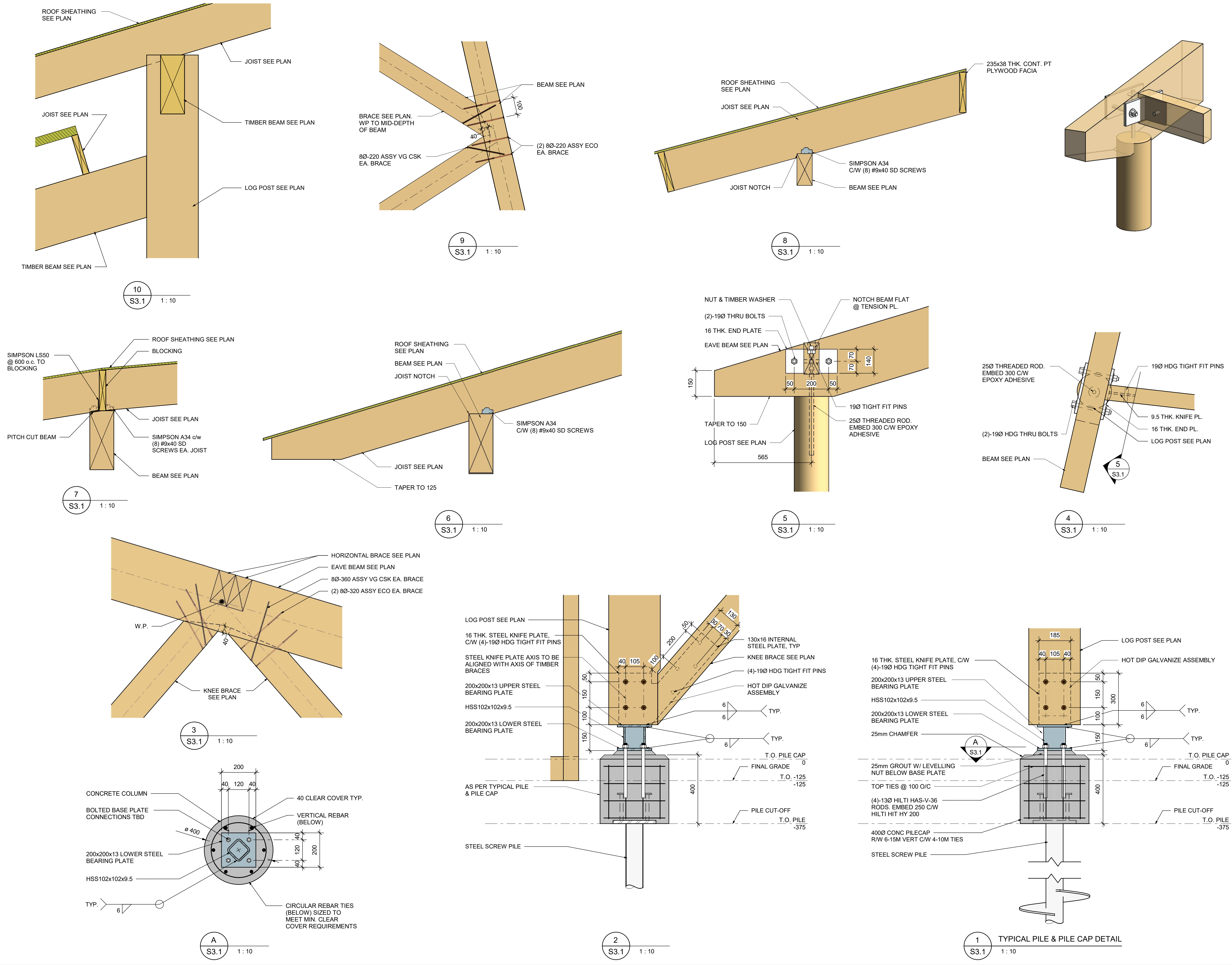
Title

**BUILDING
SECTIONS**

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S3.0

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Title
SECTIONS & DETAILS

Scale	1 : 10	Revision No.	2
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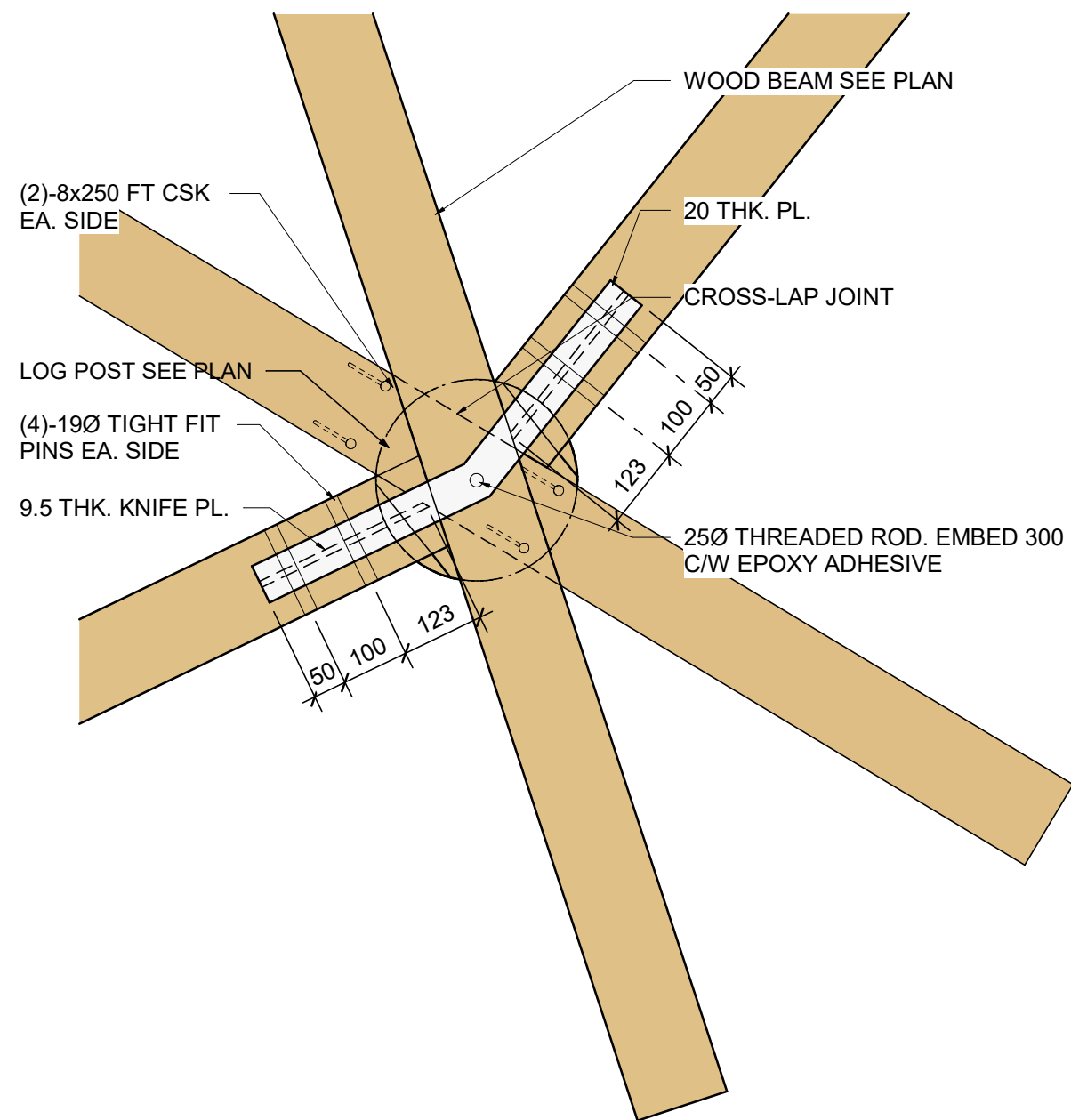
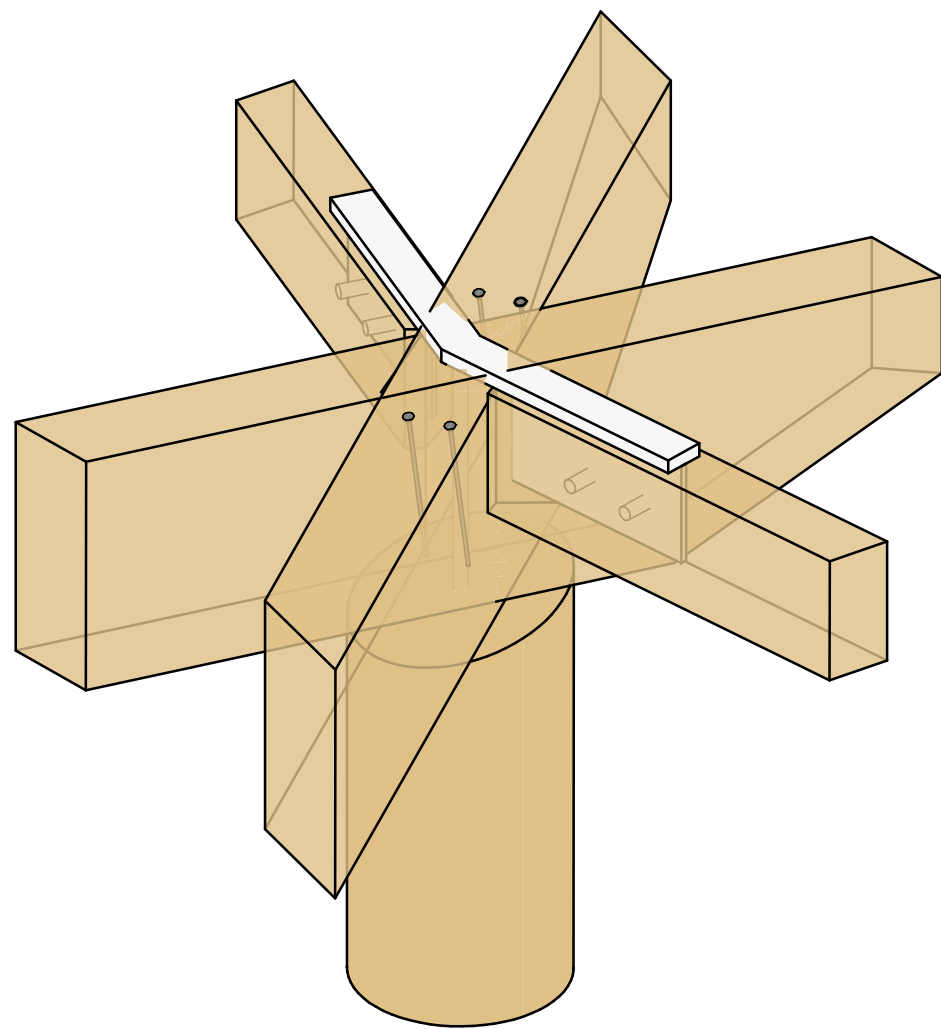
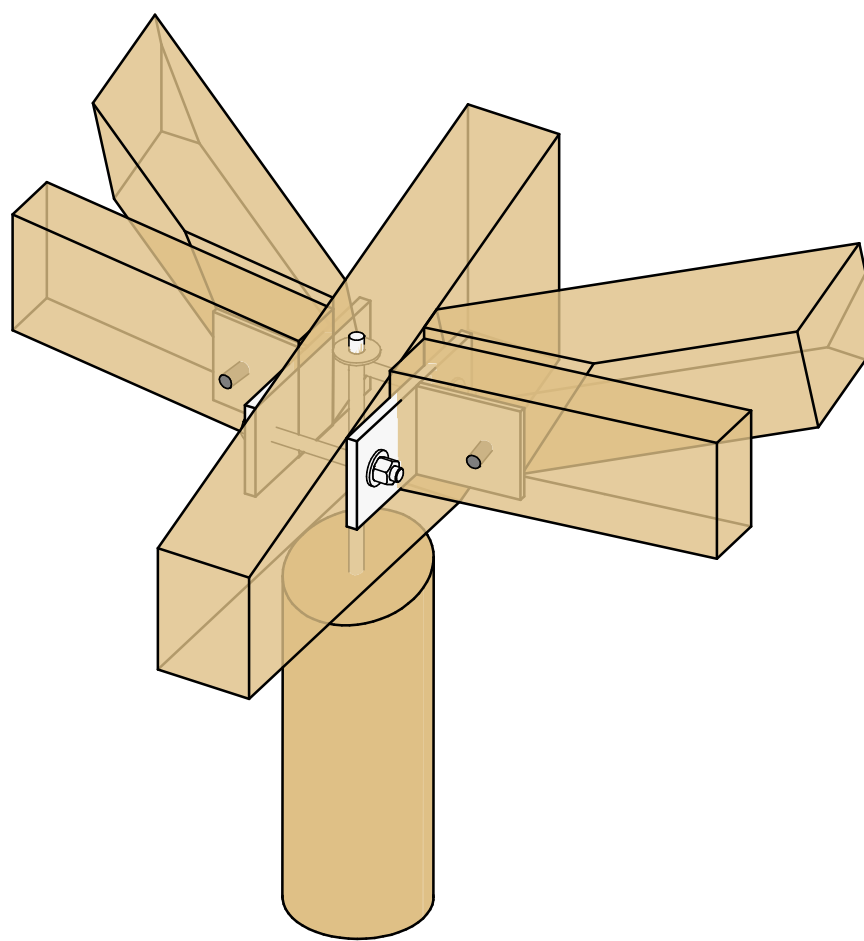
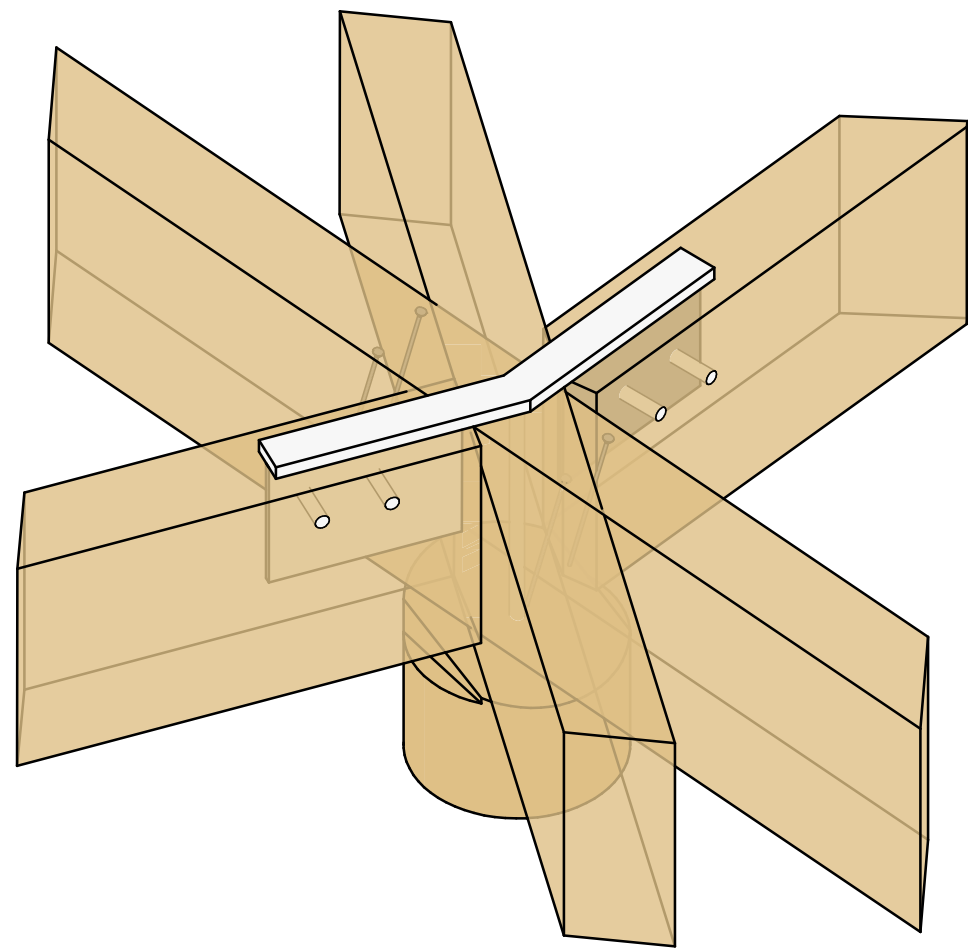
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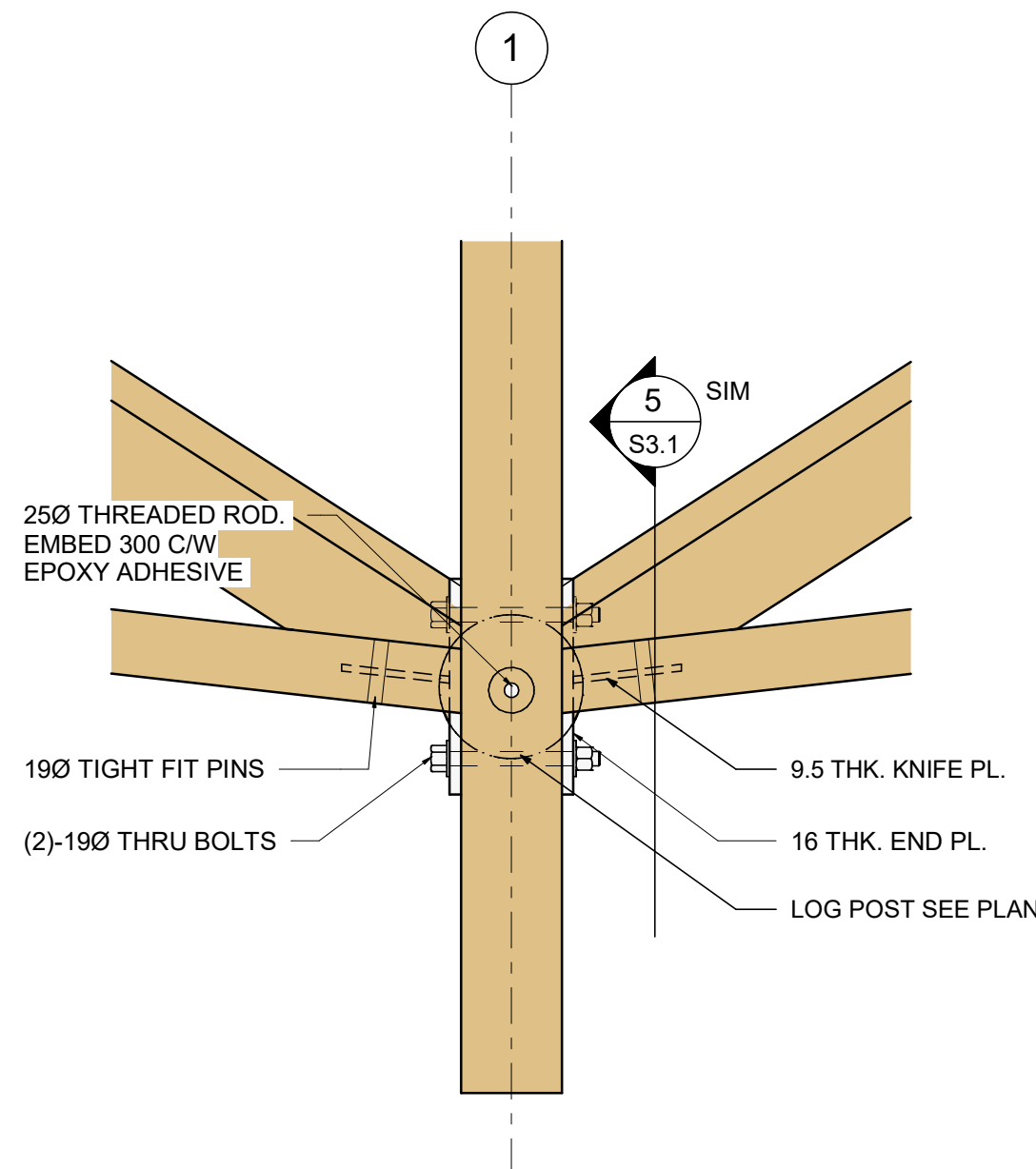
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SECTIONS & DETAILS

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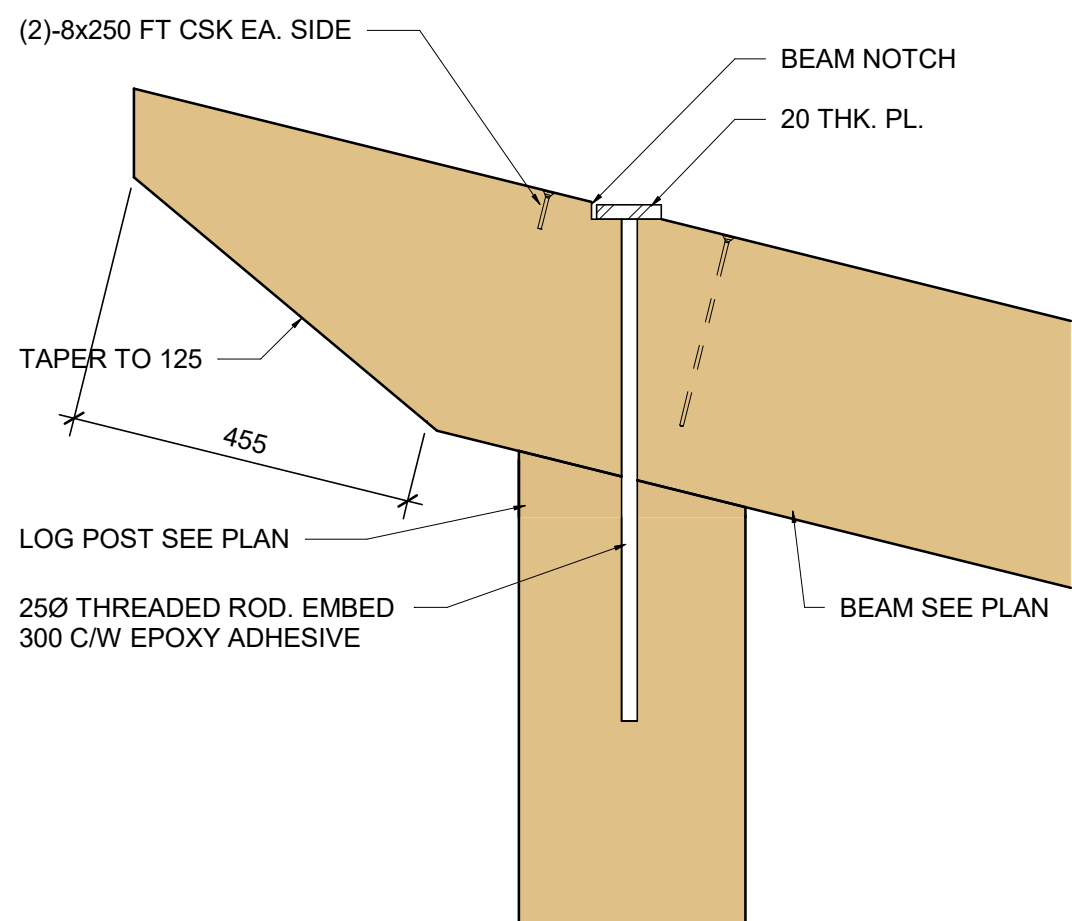
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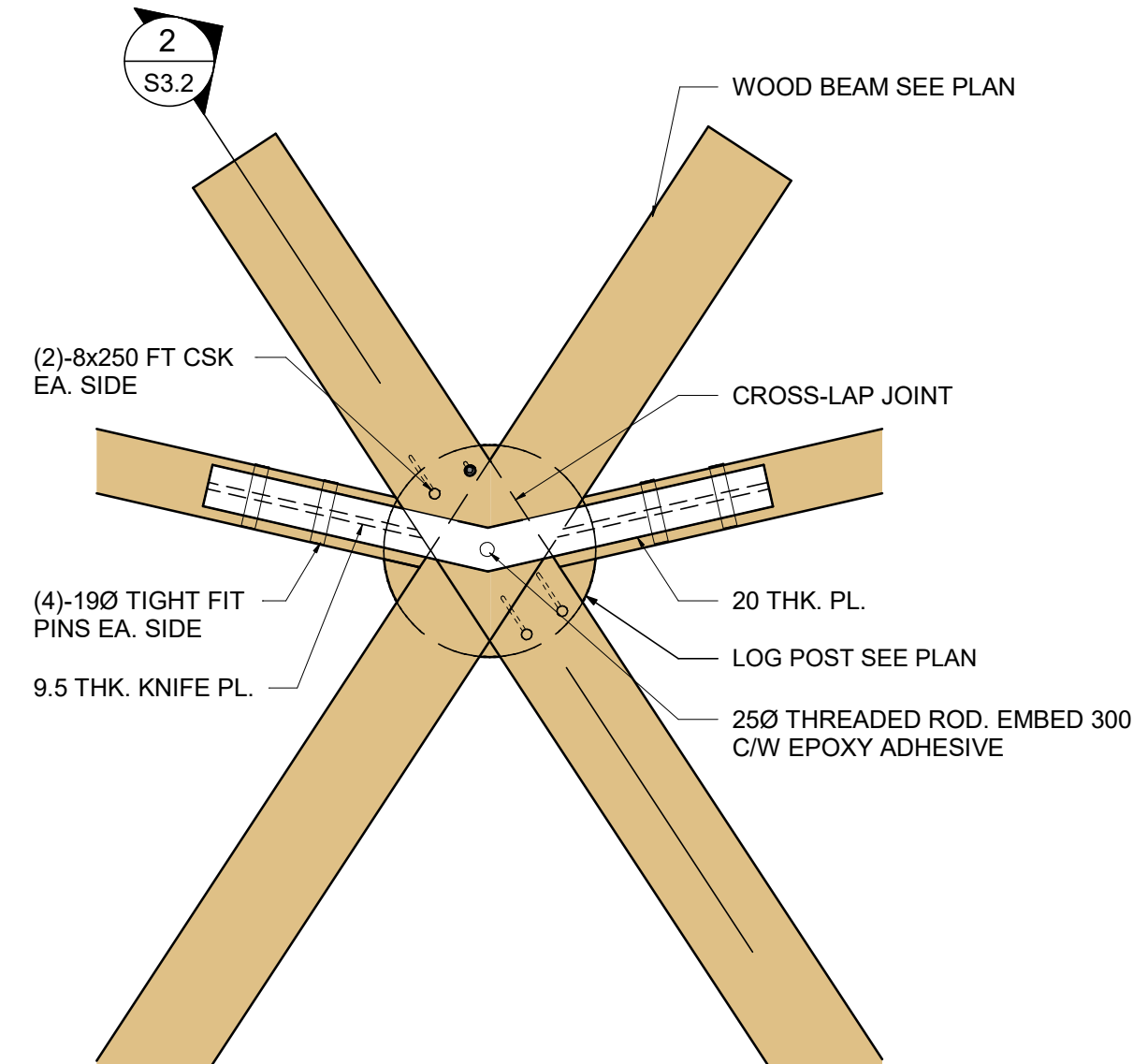
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S3.2 1 : 10



3
S3.2 1 : 10

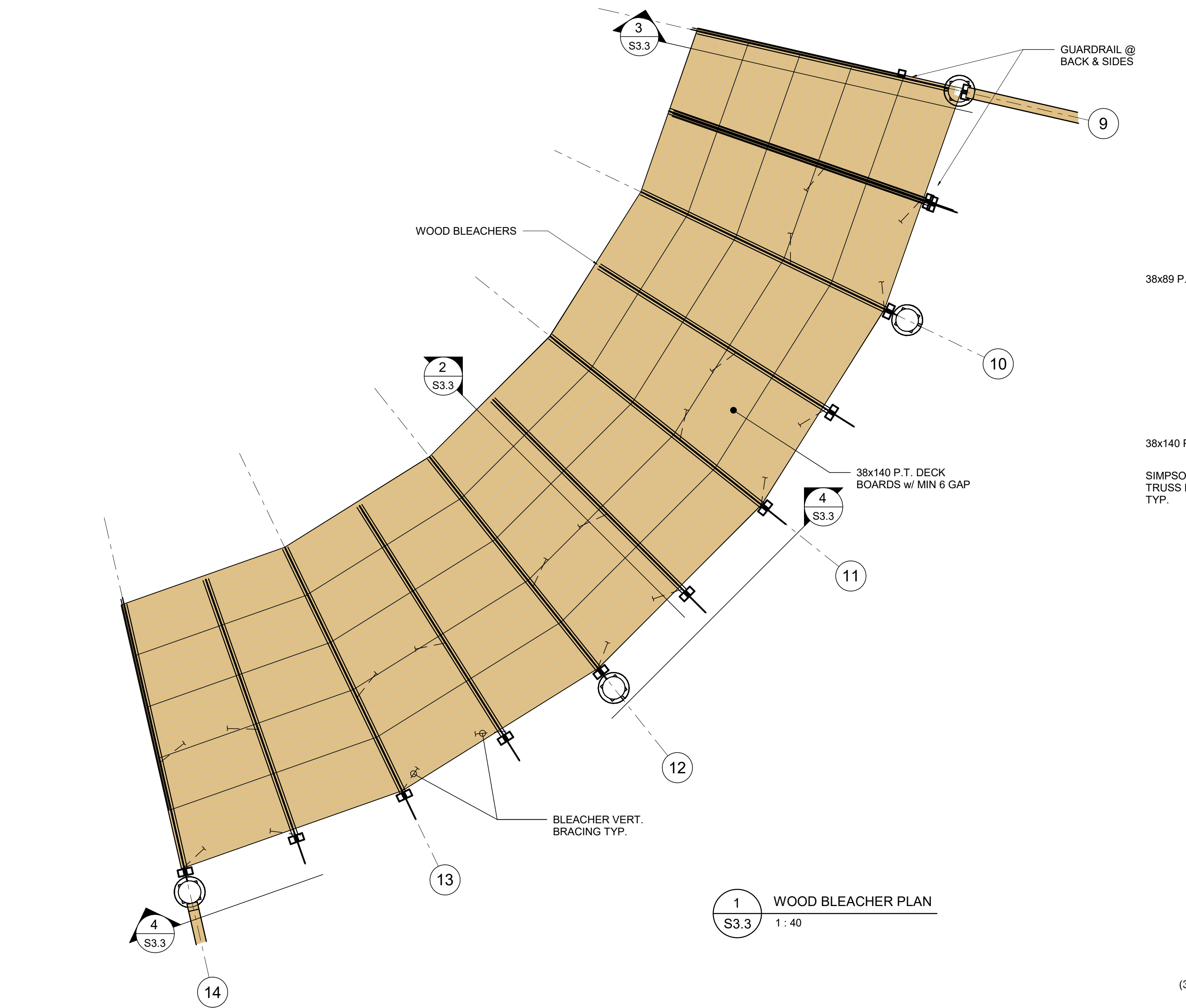


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S3.2 1 : 10

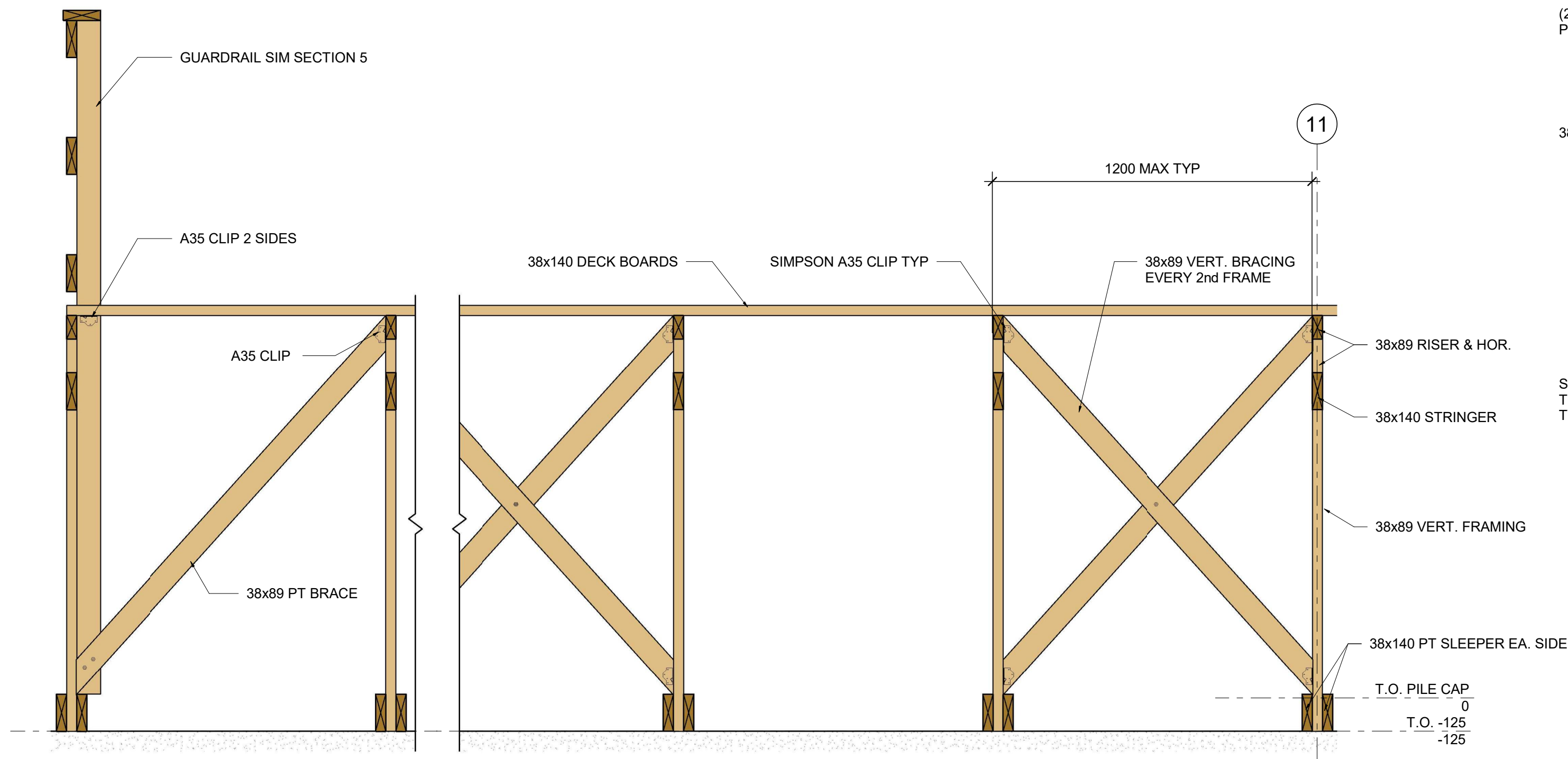


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S3.2 1 : 10

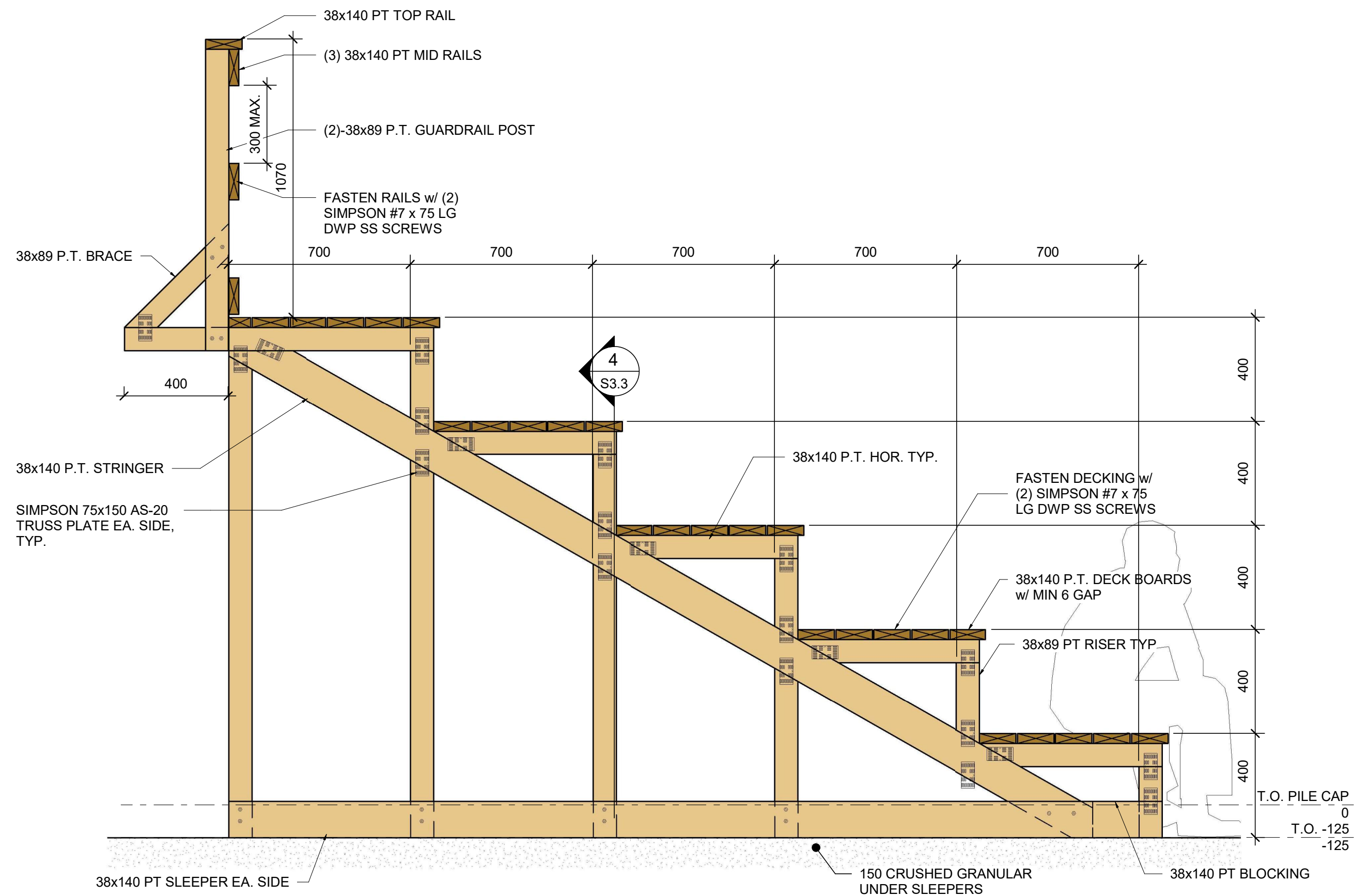
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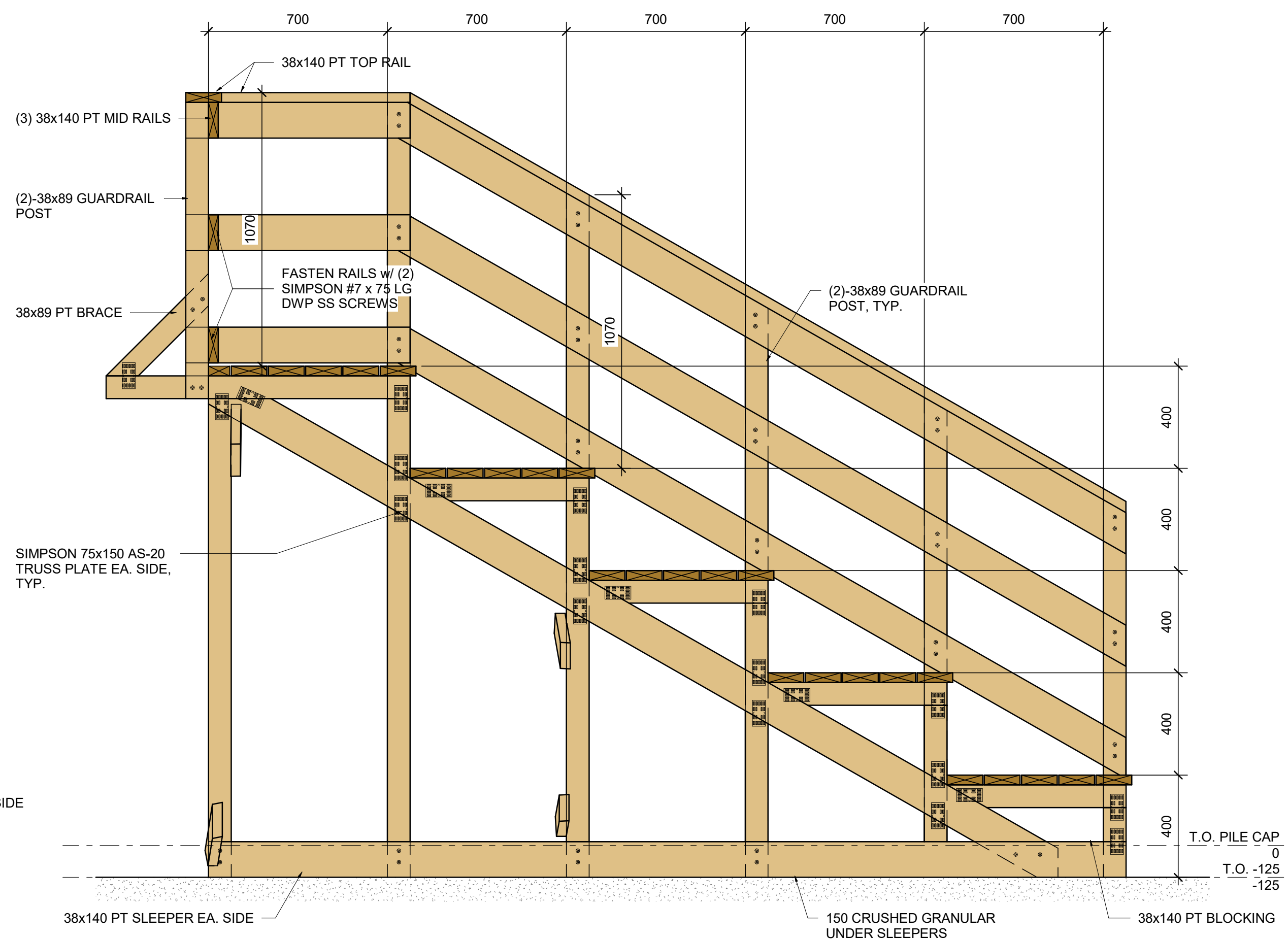
1 WOOD BLEACHER PLAN
S3.3 1:40



4
S3.3 1:15



2 TYPICAL BLEACHERS
S3.3 1:15



3 END OF BLEACHERS SECTION
S3.3 1:15



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PRINCE ALBERT, SASKATCHEWAN

Title
**WOOD BLEACHER
PLANS &
SECTIONS**

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S3.3